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**NARROMINE SHIRE COUNCIL**  
**ORDINARY MEETING BUSINESS PAPER – 10 NOVEMBER 2021**  
**REPORTS TO COUNCIL – INFRASTRUCTURE AND ENGINEERING SERVICES**

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**1. WORKS REPORT**

<b>Author</b>	Director Infrastructure and Engineering Services
<b>Responsible Officer</b>	Director Infrastructure and Engineering Services
<b>Link to Strategic Plans</b>	CSP – 4.3.4 Ensure Council's property assets are monitored and well managed

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**Executive Summary**

This report provides information regarding works undertaken for the given period in regards to both operational and capital works.

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**Report**

The Works Report (**Attachment No. 1**) for the period 7 October 2021 to 3 November 2021 is presented to Council for information.

**Financial Implications**

Council has provision for these services in its Operational Budget.

**Legal and Regulatory Compliance**

Local Government Act 1993  
Roads Act 1993

**Risk Management Issues**

Nil

**Internal/External Consultation**

Nil

**Attachments**

1. Works Report

**RECOMMENDATION**

That the information be noted.

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## **2. SHORT TERM LEASE OF COUNCIL ROAD RESERVE**

<b>Author</b>	Manager Engineering Services
<b>Responsible Officer</b>	Director Infrastructure and Engineering Services
<b>Link to Strategic Plans</b>	CSP – 4.3.4 Ensure Council's property assets are monitored and well managed.

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### **Executive Summary**

The purpose of this report is for Council to consider and adopt a fee for the lease of various sections of the road reserve on the Sessian Road, Trangie.

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### **Report**

Council has been approached by Robinson Grain Trading to lease various portions of road reserve along Sessian Road, for their internal operations. Currently, Robinson Grain Trading are using portions of the road reserve to load containers onto the rail network.

Robinson Grain Trading is requesting to lease one portion of road reserve to use as laydown areas for access to the rail network associated with the various operations. The total land area that would be used is approximately 4.44ha. The portion is located from Westbury Road to the southeast for approximately 1.11km. The lease area will be for the frontage of Lot 41 DP 1176676, (**Attachment No. 2**).

Robinson Grain Trading have previously indicated that they would like to purchase the land, however, it was advised that the road reserve is of significance to Council's future plans, in terms of access.

Council does not have set fees for the lease of road reserves in its Fees and Charges. It is recommended that the lease fee be set for \$5,000 (excl GST) per annum.

Council may lease land comprising a public road (other than a Crown road) to the owner or lessee of land adjoining the public road if, in its opinion, the road is not being used by the public.

Public notice will be given in terms of the Roads Act, 1993, once Council has considered and adopted the fee.

Council will enter into a formal lease agreement with Robinson Grain Trading that will set out terms and conditions after adoption of the fees and after public notice.

### **Financial Implications**

Income for the lease of a road reserve

### **Legal and Regulatory Compliance**

Local Government Act 1993.

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**2. SHORT TERM LEASE OF COUNCIL ROAD RESERVE CONT.**

Roads Act, 1993.

**Risk Management Issues**

The road reserve will be managed by agreement between Council and Robinson Grain Trading.

**Internal/External Consultation**

Consultation with Planning Manager

Public notice as part of the Roads Act 1993, requirements.

**Attachments**

1. Map indicating proposed lease area

**RECOMMENDATION**

1. That Council consider and adopt the proposed fee of \$5,000 for the lease of the road reserves along Sessian Road.
  2. That the fee be reviewed annually.
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**3. NARROMINE STORMWATER DRAINAGE STUDY REPORT**

<b>Author</b>	Infrastructure and Engineering Services Director
<b>Responsible Officer</b>	Infrastructure and Engineering Services Director
<b>Link to Strategic Plans</b>	CSP - 2.1.5 New Plans and Strategies are developed in line with the community's needs and encourage economic growth CSP - 2.2.6 Planning mechanisms that support the provision of suitable and serviceable land that will support infrastructure that allows for localised employment opportunities CSP – 3.1.2 Review Council facilities and activities to minimise environmental impact CSP 3.3.2 Ensure development needs align to utilities infrastructure

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**Executive Summary**

The purpose of this report is to seek Council's endorsement to adopt the Narromine Stormwater Drainage Study Report.

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**ORDINARY MEETING BUSINESS PAPER – 10 NOVEMBER 2021**  
**REPORTS TO COUNCIL – INFRASTRUCTURE AND ENGINEERING SERVICES**

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**3. NARROMINE STORMWATER DRAINAGE STUDY REPORT CONT.**

**Report**

Council will recall that at its Ordinary Meeting on 16 June 2021, it was resolved that the draft Narromine Stormwater Drainage Study Report (**Attachment No. 3**) be adopted and placed on public exhibition for a period of 28 days. (Resolution 2021/132)

The draft Narromine Stormwater Drainage Study Report was placed on Council's website and Facebook, as well as hard copies distributed to 4 locations within the Shire for public viewing. Submissions closed on 16 July 2021.

One submission was received which was attached under separate cover in the August 2021 Council Business Paper. Respondents discussed issues at the Public Forum and Council resolved to defer the item until a physical site inspection was conducted (Resolution 2021/172). A summary of their issues as well as a response to their concerns is provided in the table below.

**Community Submissions**

<b>Issue</b>	<b>Detail of Concern Raised</b>	<b>Staff Comment</b>
Location and alignment of proposed drainage line running through properties	<p>a) The untitled plan on page 2 as presented by Council results in the drainage line crossing over Barlow's Lane and then into our Client's property and also into those of their neighbour's.</p> <p>b) It is not understood why the drainage path cannot be altered to extend along and be contained within the existing Barlow Lane roadway easement, subsequently discharging directly into the Macquarie River.</p>	<p>a) The mentioned Barlow's Lane is not a Council owned asset and is therefore considered to be a private road. Required processes and procedures would be followed during the detail design and feasibility studies, which will include environmental studies.</p> <p>b) The alignment will be reviewed during the detailed design stage to determine the optimum route and required structures (i.e. open drain or underground pipes), which could include the conveyance of stormwater in a westerly direction to the vicinity of the proposed Basin 1.</p>

A site visit is planned for 10 November 2021 whereby Councillors will have the opportunity to gain an understanding of the concerns raised, as well as a solution(s) to those concerns.

While this Stormwater Drainage Study Report identifies stormwater treatment options at a strategic level, further specialist studies and investigations such as environmental, geotechnical, hydraulic and financial would be required during the implementation

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**3. NARROMINE STORMWATER DRAINAGE STUDY REPORT CONT.**

of this strategic document. Land use planning for development will also be considered to mitigate stormwater impacts to the community and infrastructure. A staged approach will also be followed during the implementation of this strategy.

**Financial Implications**

The Narromine Stormwater Drainage Study Report will be used to develop future capital works and improvements programs, for stormwater systems in Narromine.

**Legal and Regulatory Compliance**

Local Government Act 1993

Roads Act 1993

Protection of the Environment Operations Act, 1997

**Risk Management Issues**

A staged approach will be implemented for improved stormwater management and environmental controls.

An adopted stormwater strategy for provides direction for future development.

**Internal/ external Consultation**

Consultation has been undertaken with the Narromine Community/Stakeholders. Further consultation may be required during implementation of stormwater treatment controls and during the detailed design.

Public forum on 25 August 2021

**Attachments**

1. Narromine Shire Council Stormwater Drainage Study Report

**RECOMMENDATION**

That Council adopt the Narromine Stormwater Drainage Study Report, considering that a staged approach to its implementation will be followed.

André Pretorius  
**Director Infrastructure and Engineering Services**

**Attachment No. 1**

 <b>Narromine</b> <small>SHIRE COUNCIL</small>	<b>MONTHLY WORKS REPORT</b> <b>Wednesday, 3 November</b> <b>2021</b>	Infrastructure and Engineering Services Narromine Shire Council Tel: 02 6889 9999 Fax: 02 6889 9998 <a href="mailto:mail@narromine.nsw.gov.au">mail@narromine.nsw.gov.au</a>
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Road and Park users are to proceed with caution at all work sites and observe work signs to ensure safety. Speed zones are enforceable with possible short delays. For all enquiries, please contact Council's Infrastructure and Engineering Services Department on 6889 9999.
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<b>URBAN ROADS – Narromine, Trangie, Tomingley</b>	
<b>Various Streets (Narromine)</b>	<u><b>Maintenance:</b></u> <ul style="list-style-type: none"> <li>• Routine Maintenance Program such as patching and street sweeping, etc.</li> <li>• Weed spraying of streets, lanes, gutters and footpaths ongoing.</li> <li>• Weed spraying of the pump stations, storm water drains and water towers completed.</li> <li>• Maintenance of the laneways continuing.</li> <li>• Shoulder maintenance and sealing complete in Minore Street.</li> </ul>
<b>Various Streets (Trangie)</b>	<u><b>Maintenance:</b></u> <ul style="list-style-type: none"> <li>• Routine Maintenance Program such as patching and street sweeping, etc.</li> <li>• Replacement of Footpath at Derribong Street, Trangie Central School completed.</li> <li>• New drainage works will commence when dry enough in Enmore Street and Mullah Street.</li> <li>• Finalising designs for the intersection of drainage crossing at Enmore and Mungery Street.</li> </ul>
<b>Various Streets (Tomingley)</b>	<u><b>Maintenance:</b></u> <ul style="list-style-type: none"> <li>• Routine Maintenance Program by contractor.</li> </ul>
<b>UNSEALED ROADS NETWORK</b>	
<b>Various Unsealed Roads</b>	<u><b>Maintenance: Map No. 1</b></u> <ul style="list-style-type: none"> <li>• Pineview Road</li> <li>• Jamea Lane</li> </ul> <u><b>Capital: Storm/Flood Funding repair Map No. 2</b></u> <ul style="list-style-type: none"> <li>• Belmont Road</li> <li>• Lockwood Road</li> <li>• Wyanga Road</li> <li>• Alison's Road</li> <li>• Pineview Road</li> <li>• Merrinong Road</li> </ul> <u><b>Roadside spraying completed by Council - Biosecurity</b></u> <ul style="list-style-type: none"> <li>• Tantitha Road</li> <li>• Brumagen Road</li> <li>• Dappo Road</li> <li>• Cowal Park Road</li> <li>• Kyalite Road</li> </ul>
<b>SEALED ROADS NETWORK</b>	
<b>Various Sealed Roads</b>	<u><b>Maintenance:</b></u> <ul style="list-style-type: none"> <li>• Pothole patching where required.</li> </ul>

**Attachment No. 1**

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**SEALED ROADS NETWORK CONT.**

<b>Various Sealed Roads cont.</b>	<u><b>Emergency Heavy Patching</b></u> <ul style="list-style-type: none"> <li>• Eumungerie Road</li> <li>• Tomingley Road</li> <li>• Frecklington's Crossing</li> </ul> <u><b>Capital:</b></u> <ul style="list-style-type: none"> <li>• Willydah Road – Rahab continuing</li> </ul>
	<u><b>Roadside spraying completed by Council - Biosecurity</b></u> <ul style="list-style-type: none"> <li>• Newell Highway</li> <li>• Dandaloo Road</li> <li>• Trangie Collie Road</li> <li>• Newell Highway</li> <li>• Peak Hill Railway Road</li> <li>• Warren Road</li> <li>• Cathundral Road</li> </ul>

**SWIMMING POOLS**

<b>Narromine Pool</b>	<ul style="list-style-type: none"> <li>• Open for the season.</li> </ul>
<b>Trangie Pool</b>	<ul style="list-style-type: none"> <li>• Open for the season.</li> </ul>

**PARKS AND OPEN SPACE NETWORK CBD Gardens, Parks, Ovals, Villages**

<b>Narromine CBD</b>	<ul style="list-style-type: none"> <li>• Ongoing vegetation management.</li> <li>• Staged reduction of hedge height in main street ongoing.</li> </ul>
<b>Narromine Parks and Reserves</b>	<ul style="list-style-type: none"> <li>• General maintenance and mowing.</li> <li>• Weed control in all small parks ongoing.</li> <li>• Signs erected advising the parks are not cleaned on a regular basis.</li> <li>• Dundas Park Playground construction is nearing completion. Rubber installation has commenced with the park to be opened by week ending 5 November 2021.</li> <li>• Wetlands – Construction of primary pond continuing. Additional drainage structures being installed, and the construction of foot paths has commenced.</li> </ul>
<b>Narromine Sports Grounds</b>	<ul style="list-style-type: none"> <li>• General maintenance, spraying and mowing.</li> <li>• Apex Park netball courts – Fitting out of the canteen and toilets is ongoing. Parking area on Terangion Street now complete.</li> <li>• Cale oval – demolition of the old grandstand has been completed, and construction of the new grandstand has commenced.</li> </ul>
<b>Narromine Streets</b>	<ul style="list-style-type: none"> <li>• General maintenance.</li> <li>• Street sweeping continues.</li> </ul>
<b>Trangie CBD</b>	<ul style="list-style-type: none"> <li>• General maintenance and weed control ongoing.</li> <li>• Vegetation mowing and slashing continuing.</li> </ul>

**Attachment No. 1**

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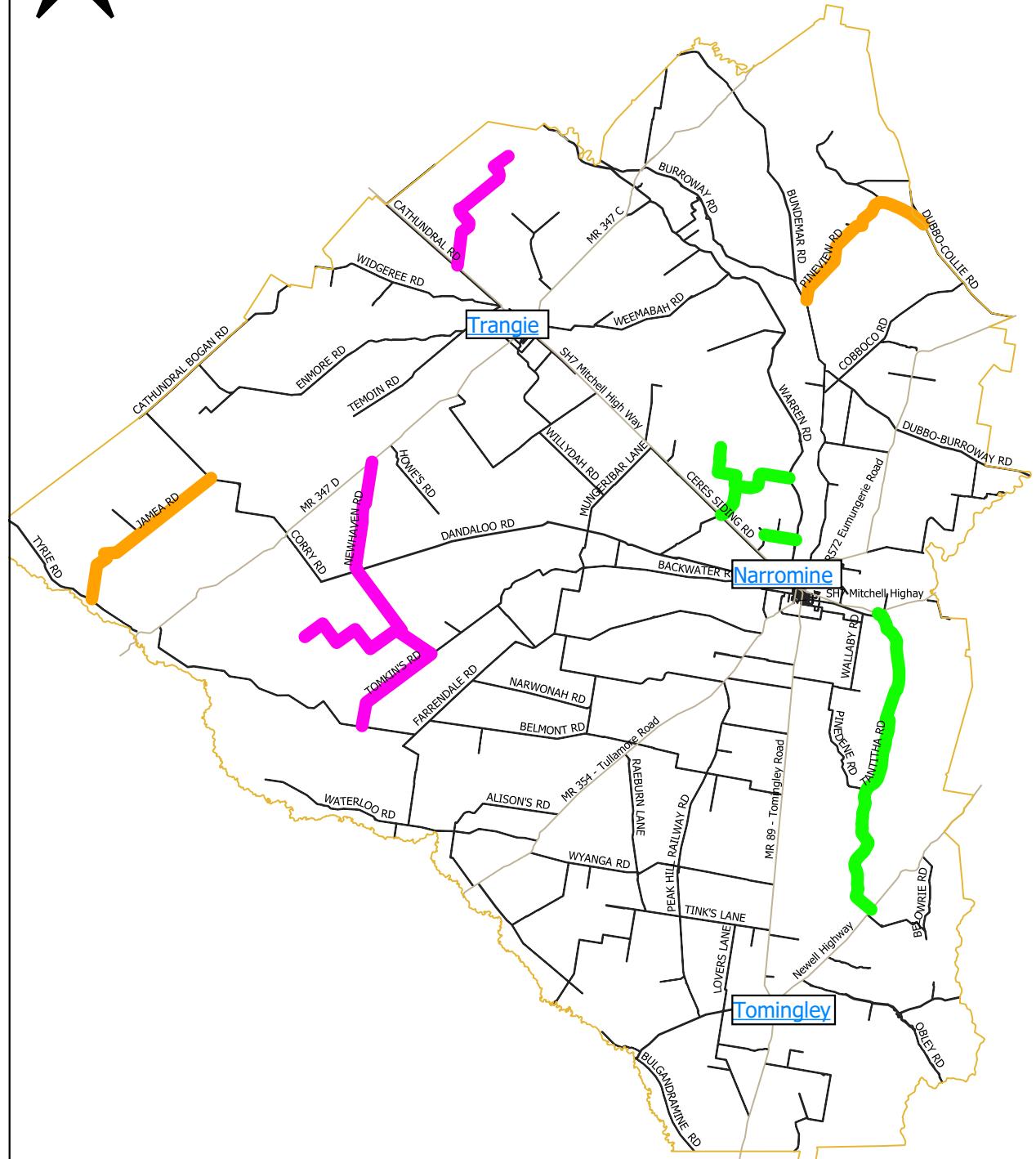
<b>PARKS AND OPEN SPACE NETWORK CBD Gardens, Parks, Ovals, Villages CONT.</b>	
<b>Trangie Parks</b>	<ul style="list-style-type: none"> <li>General maintenance and mowing.</li> <li>Signs erected advising the parks are not cleaned on a regular basis.</li> </ul>
<b>Trangie Sports Grounds</b>	<ul style="list-style-type: none"> <li>Maintenance program being amended due to change in season.</li> <li>Automatic lock mechanisms have been installed into the public toilets at Burns Oval to allow for public use, 10am - 6pm.</li> </ul>
<b>Trangie Streets</b>	<ul style="list-style-type: none"> <li>General maintenance.</li> <li>Street sweeping weekly on Thursdays.</li> </ul>
<b>Tomingley Village</b>	<ul style="list-style-type: none"> <li>Position of the new multipurpose court has been marked out and earthworks have now commenced.</li> </ul>
<b>AERODROME</b>	
<b>Narromine Aerodrome</b>	<ul style="list-style-type: none"> <li>Runway Lighting Upgrade has commenced</li> <li>Tree trimming, slashing and weed control continues.</li> </ul>
<b>BUILDING MAINTENANCE</b>	
<b>All Buildings</b>	<ul style="list-style-type: none"> <li>General maintenance as required.</li> </ul>
<b>Vandalism</b>	<ul style="list-style-type: none"> <li>Graffiti continues in both Narromine and Trangie.</li> </ul>
<b>Narromine Medical Centre</b>	<ul style="list-style-type: none"> <li>General maintenance as required.</li> </ul>
<b>Council Administration Buildings</b>	<ul style="list-style-type: none"> <li>General maintenance as required.</li> </ul>
<b>PUBLIC CONVENIENCES</b>	
<b>Rotary Park (Narromine) Public Toilets</b>	<ul style="list-style-type: none"> <li>Toilet facilities cleaned daily, and QR Codes installed.</li> </ul>
<b>Burraway Street Public Toilets (adjacent to Pool)</b>	<ul style="list-style-type: none"> <li>Toilet facilities cleaned daily, and QR Codes installed.</li> </ul>
<b>Manildra Street Toilets (at Saleyards)</b>	<ul style="list-style-type: none"> <li>Toilet facilities are open, 24 hours a day, 7 days a week and are cleaned daily, and QR Codes installed.</li> </ul>
<b>Argonauts Park (Trangie) Public Toilets (Goan Waterhole)</b>	<ul style="list-style-type: none"> <li>Toilet facilities cleaned every Tuesday, Thursday and Saturday, and QR Codes installed.</li> </ul>
<b>Dandaloo Street Trangie (adjacent to Bakery)</b>	<ul style="list-style-type: none"> <li>Toilet facilities cleaned every Monday, Wednesday and Friday</li> <li>QR Codes installed.</li> </ul>
<b>Burns Oval Toilets</b>	<ul style="list-style-type: none"> <li>Toilet facilities cleaned every Monday, Wednesday and Friday</li> <li>QR Codes to be installed.</li> </ul>
<b>Trangie Tuck Stop Toilets</b>	<ul style="list-style-type: none"> <li>Checked daily with a main clean every Tuesday and Thursday and either late Saturday or Sunday morning over weekends, and QR Codes installed.</li> </ul>
<b>Trangie Truck Wash</b>	<ul style="list-style-type: none"> <li>Tenders closed and tender evaluation in process.</li> </ul>

**Attachment No. 1**

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<b>PUBLIC CONVENIENCES CONT.</b>	
<b>Trangie Truck Wash cont.</b>	<ul style="list-style-type: none"> <li>Stakeholders will be consulted as part of the design and construct process, once a contractor is appointed.</li> </ul>
<b>Wetlands</b>	<ul style="list-style-type: none"> <li>Toilet block secured and cleaned daily, and QR Codes installed</li> </ul>
<b>CEMETERIES</b>	
<b>Narromine Cemetery</b>	<ul style="list-style-type: none"> <li>General maintenance, mowing and weed spraying.</li> <li>Topping up of subsiding graves continues.</li> </ul>
<b>Trangie Cemetery</b>	<ul style="list-style-type: none"> <li>General maintenance, mowing and weed spraying.</li> <li>Topping up of subsiding graves continues.</li> </ul>
<b>WATER AND SEWER</b>	
<b>Tomingley</b>	<ul style="list-style-type: none"> <li>Regular maintenance at the water treatment plant.</li> <li>Staff and contractors have completed earthworks and the building is in place for the new water treatment plant.</li> </ul>
<b>Trangie</b>	<ul style="list-style-type: none"> <li>Staff have continued reticulation system maintenance.</li> <li>Staff continue regular sewer pump station and STP system maintenance as required.</li> <li>Staff continue daily operational Drinking Water Quality Testing as required by legislation.</li> <li>Staff replaced 1 house water services due to repeated leaks and severe corrosion.</li> <li>Staff have collected sewage samples for the NSW Health Department COVID 19 Sewage Surveillance Project.</li> <li>Staff have continued replacement of consumer water meters that are not registering or are over 15 years of age.</li> </ul>
<b>Narromine</b>	<ul style="list-style-type: none"> <li>Staff continue daily operational Drinking Water Quality Testing as required by legislation.</li> <li>Staff continue reticulation system maintenance.</li> <li>Staff have continued the regular service maintenance programs for the Narromine sewage pumping station network.</li> <li>Staff have continued replacement of consumer water meters that are not registering or are over 15 years of age.</li> <li>Staff have collected sewage samples for the NSW Health Department COVID 19 Sewage Surveillance Project.</li> </ul>

Map No. 1



0 10 20 km

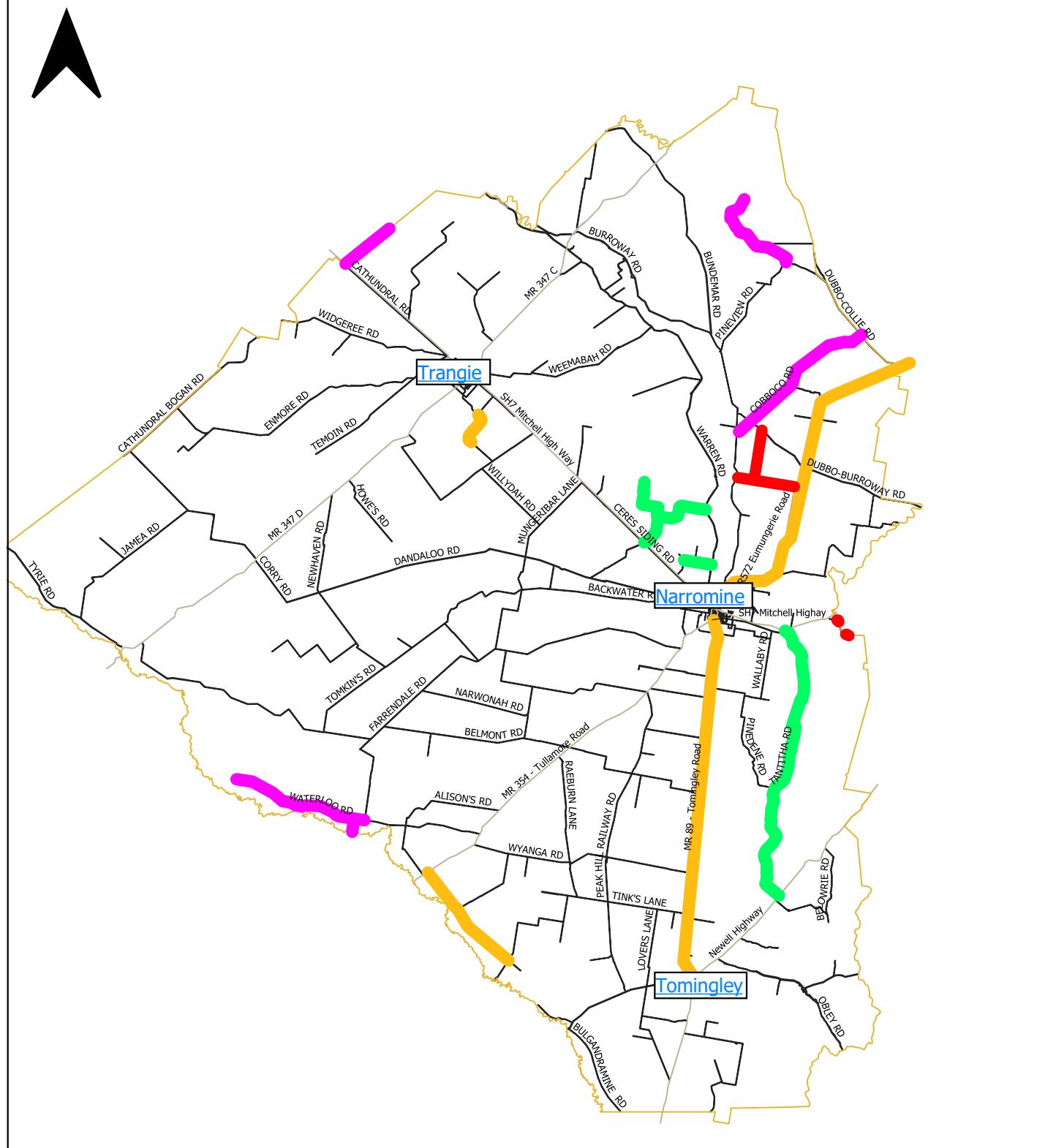
## Narromine Shire Council

June	February	October
May	January	September
April	December	August
March	November	July



Note - the roads highlighted are visual purposes only and may not be the exact location of work

Map No. 2



0 10 20 km

## Narromine Shire Council

June	February	October
May	January	September
April	December	August
March	November	July



Note - the roads highlighted are visual purposes only and may not be the exact location of work

## Attachment No. 2

Created on 28/10/2021 2:40 PM



### Important Notice!

This map is not a precise survey document. Accurate locations can only be determined by a survey on the ground.

This information has been prepared for Council's internal purposes and for no other purpose. No statement is made about the accuracy or suitability of the information for use for any purpose (whether the purpose has been notified to Council or not). While every care is taken to ensure the accuracy of this data, neither the Narromine Shire Council nor the SS makes any representations or warranties, express or implied, about the accuracy, reliability, or completeness of the data for any particular purpose and disclaims all responsibility and all liability (including, without limitation, liability in negligence) for all expenses, losses, damages (including indirect or consequential damage) and costs which you might incur as a result of the data being inaccurate or incomplete in any way and for any reason.

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Projection: GDA94 / MGA zone 55

Date: 28/10/2021

Drawn By: Jordan Richardson

Map Scale: 1:18009 at A4



Our Reference: 2111

**Catchment Survey & Analysis  
Narromine, NSW**

**For  
Narromine Shire Council**

**Stormwater Drainage Study Report**

**Prepared by:**

**Craig & Rhodes Pty Ltd T/A Storm Consulting**

**Suite 7.01, Level 7**

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DOCUMENT CONTROL				
Revision	Date	Comment	Prepared	Reviewed
A	03.09.20	Draft Report	JK	DD
B	02.02.21	Final Report (Council Comments)	JK	RK

## 1 Introduction

Storm Consulting has been engaged by Narromine Shire Council to undertake the data collection within the drainage catchments, report on the hydrology and hydraulics of the existing stormwater drainage system and to provide options to alleviate flooding in key flood prone areas within the Narromine Township located approximately 40km west of Dubbo in the Orana region, NSW.

Previous Storm Consulting study '*Drainage Feasibility Study of the Narromine Northern Catchment (Rev A)*' dated 21/12/18 indicated many areas of the town are flood prone due to undersized stormwater network located on extremely flat terrain. The study also provided a hydrologic and hydraulic assessment of the main Northern Catchment. This report focuses on the remaining township catchments not covered in the previous study.

The objectives of the study are:

- Review of data including the detailed survey by Craig&Rhodes and flood study reports received from Council
- Analyse the drainage system under existing conditions for 1yr, 10yr, 20yr, 50yr and 100yr ARI including peak flows, volumes and velocities
- identify drainage options to improve existing drainage capacity in accordance with major/minor drainage philosophy for:
  - 5-year Average Recurrence Interval (ARI) / 20% Annual Exceedance Probability (AEP)
- provide a cost estimate for the proposed work for each option presented.

## 2 Available data

Data available and reviewed for the study are as following:

- Council GIS layers including catchment extent, pipes, channel and culvert layers. This data provides includes on pipe/channel invert but limited information on pit type, size and grate elevation.
- Survey information received from Council (29/06/2018 & 25/07/2018). This information is provided in Appendix C.
- Existing western channel and proposed wetland design
- Detailed Survey by Craig & Rhodes (dated 11/06/20)
- Narromine Drainage Study, PPK Consultants (1993)
- Macquarie River (Narromine to Oxley Station) Floodplain Management Plan (2008).
- Narromine Riverbank Levee Feasibility Study, Lyall & Associates (2013)

### 3 Review of Available Data

#### 3.1 Existing Landuse

The existing town landuse is largely characterised by General Residential 'single detached lot' areas (R1) and Large Lot Residential (R5) with industrial (IN1) and commercial (B1) precincts and open space areas (RE1) as presented in Figure 1.



Figure 1 Site Landzone Map (Source: NSW Planning Portal)

#### 3.2 Existing Topography

The existing surface elevation model was developed using a combination of 1m grid size LiDAR data obtained from Geoscience Australia and detailed survey showing road features (road centreline, kerb and gutter and roadside ditch drains). Existing surface elevation model showing catchment breakup in blue with overland flowpath in red is presented in Figure 2.



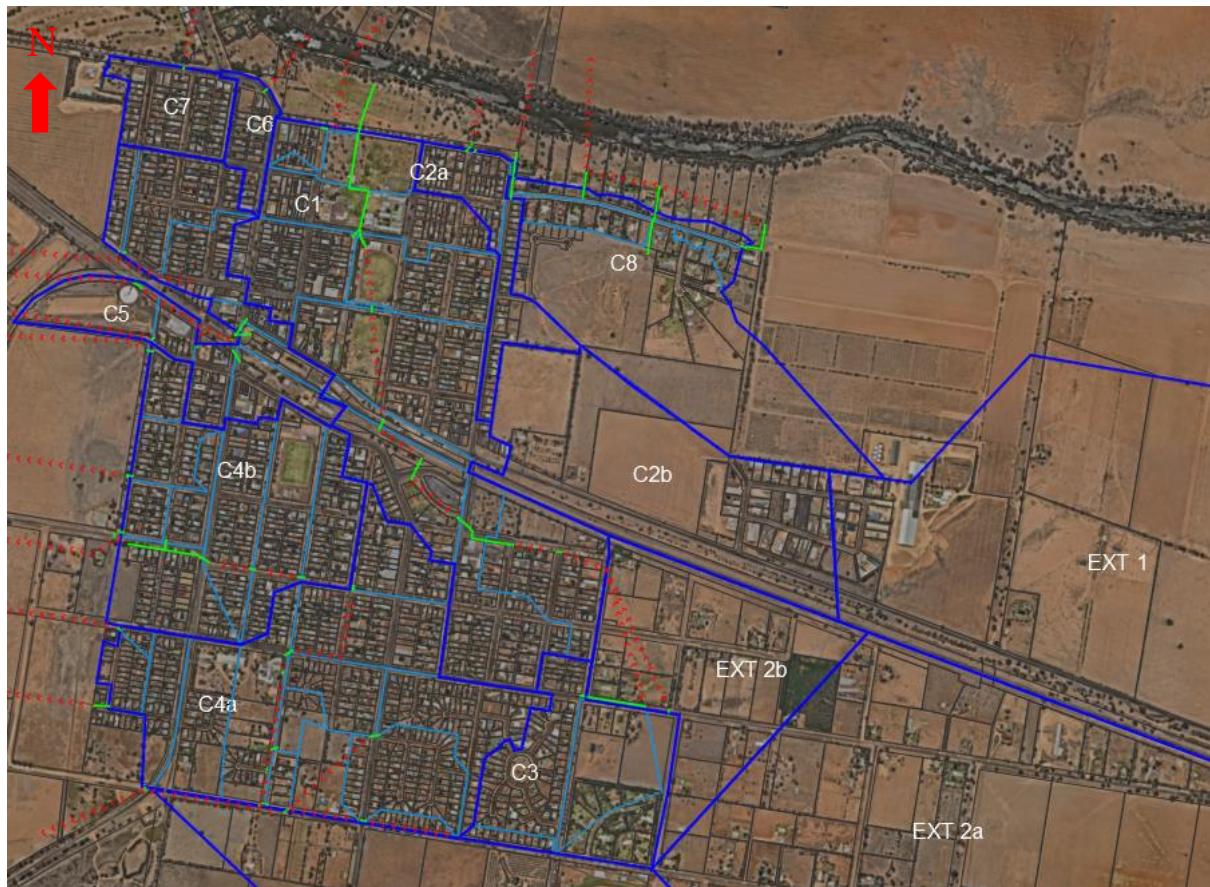
*Figure 2 Existing Topography showing existing catchments (blue) and landform overland flowpath (red)*

The surface elevation model, presented in Figure 2, shows approximately half of the township catchments draining towards Macquarie River to the North. The remaining half of the township catchments drains east and southwards towards an existing natural overland flowpath running south-west. Two large external catchments (shown in dark blue in Figure 2) drains northwards towards Macquarie River via an existing natural overland flowpath running through the middle of the town.

### 3.3 Catchment Map

Based on the landform and existing drainage network, the town consists of 9 internal catchment areas (Catchment C1 to C8) comprising of the main Narromine Township of the town as well as the eastern industrial lands and totals approximately 400ha in size. This is consistent with the mapping and labels in *Figure 1 of 'Narromine Drainage Study'* (PPK Consultants, 1993).

Existing internal catchments with catchment IDs are shown in Figure 3.



*Figure 3 Catchment Map (Catchment boundary - blue, Existing pipe – green, Overland flowpath -red)*

Two large external catchments (Catchment Ext1 and Ext2) of approximately 495 ha in size drains to Macquarie River via an existing natural channel running through the town as shown in Figure 2 and Figure 3.

The external catchments are very large in comparison to the internal town catchments with a total area of approximately 400ha.

Location and size of the catchments are presented in Figure 4.

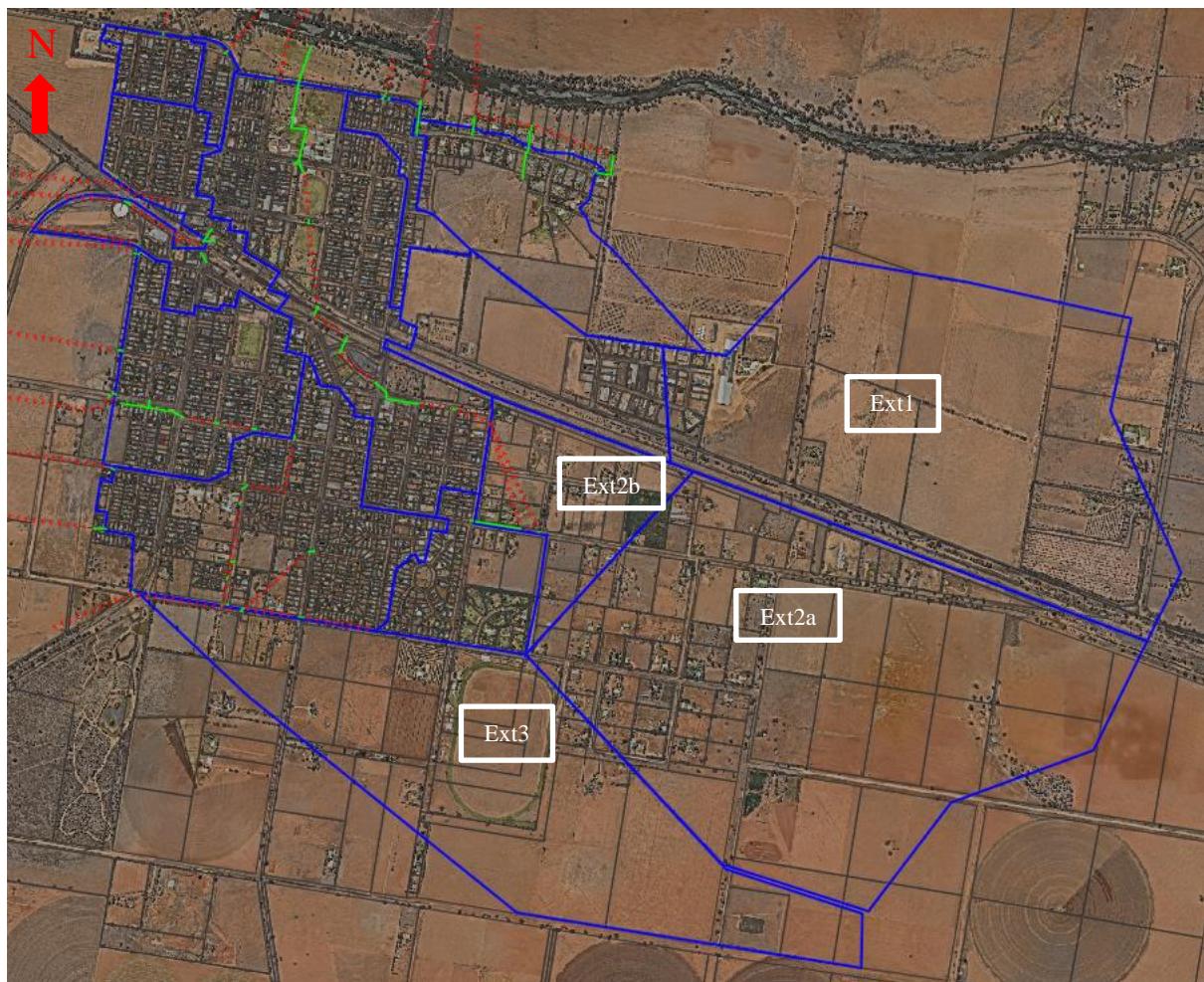


Figure 4 Catchment Map (External Catchment boundary - dark blue, Existing pipe – green, Overland flowpath - red)

Catchment summary is provided in Table 1.

Table 1 Catchment Summary

ID	Area (ha)	Max. RL	Min.RL	Approx. Slope
C1	99.8	239.5	237.5	0.20%
C2a	11.6	239	238.8	0.06%
C2b	51.2	239.2	238.6	0.08%
C3	28.6	241.4	239.5	0.36%
C4a	79.1	241.1	237.5	0.40%
C4b	48.6	239.1	238.2	0.13%
C5	38.9	239.3	238	0.21%
C6	5.1	239.1	238.5	0.27%
C7	10	239	238.1	0.28%
C8	50.5	239.9	239.4	0.07%
Ext1	203.9	246.9	238.8	0.57%
Ext2a	256.4	245.6	239.9	0.36%
Ext2b	40.7	241.5	237.8	0.58%
Ext3	163.7	243.2	237.5	0.45%

### **3.4 Existing Drainage Network**

The existing drainage network has been compiled from:

- Council GIS layers including catchment extent, pipes, channel and culvert layers. This data provides includes on pipe/channel inverts but limited information on pit type, size and grate elevation.
- Survey information received from Council (29/06/2018 & 25/07/2018). This information is provided in Appendix C.
- Existing western channel and proposed wetland design
- Detailed Survey by Craig & Rhodes (dated 11/06/20)

The existing pit and pipe information from Council's GIS system are largely characterised as having shallow grade (<0.3%) and shallow cover (<0.5m) with some drainage lines recorded as being laid against grade.

Overland flowpaths have been determined using the detailed survey by Craig & Rhodes (dated 11/06/20) which includes all local road features within the main township including kerb and gutter levels and roadside swale levels.

Rationalised assumptions of pipe sizes, pit type and invert levels have been made in areas where drainage data is unavailable or where drainage data has been assessed as erroneous.

## 4 Model Methodology

Existing and proposed hydraulic models have been developed in DRAINS (Version 2020.034) to assess the performance of the existing drainage, identify flood prone areas within the developed catchments and to provide drainage strategies to mitigate flood risks. The proposed options are discussed in detail in Section 7.

### 4.1 Hydrology

The study was undertaken in accordance with Australian Rainfall and Runoff 1987 (Pilgrim 1987). Intensity Frequency Duration (IFD) data adopted for the study area, obtained from the Bureau of Meteorology (BoM), is shown in Table 2.

*Table 2 - Intensity Frequency Duration Data – Narromine*

Intensity-Frequency-Duration Table							
Location: 32.225S 148.250E NEAR.. Meryula St Narromine Issued: 5/7/2018							
Rainfall intensity in mm/h for various durations and Average Recurrence Interval							
Average Recurrence Interval							
Duration	1 YEAR	2 YEARS	5 YEARS	10 YEARS	20 YEARS	50 YEARS	100 YEARS
5Mins	67.4	88.3	117	136	160	194	222
6Mins	62.6	82.1	109	126	149	181	206
10Mins	51.0	66.9	88.5	103	121	147	167
20Mins	37.4	48.9	64.8	75.0	88.5	107	122
30Mins	30.2	39.6	52.4	60.6	71.5	86.6	98.8
1Hr	20.0	26.1	34.5	39.9	47.0	56.9	64.8
2Hrs	12.5	16.3	21.4	24.6	29.0	35.0	39.8
3Hrs	9.32	12.2	15.9	18.3	21.4	25.8	29.3
6Hrs	5.61	7.29	9.45	10.8	12.6	15.1	17.1
12Hrs	3.38	4.39	5.67	6.48	7.56	9.04	10.2
24Hrs	2.04	2.66	3.45	3.95	4.62	5.55	6.29
48Hrs	1.19	1.57	2.08	2.37	2.80	3.38	3.85
72Hrs	.840	1.10	1.46	1.69	2.00	2.42	2.75

(Raw data: 26.99, 4.47, 1.12, 54.99, 8.6, 2.3, skew=0.34, F2=4.33, F50=15.57)

© Australian Government, Bureau of Meteorology

DRAINS standard model (ILSAX) was used to simulate the rainfall-runoff process and consider storage within the overland flow routes. The following design parameters were adopted for the analysis:

- Soil type 3 (slow infiltration),
- Depression storage – 5mm (pervious areas),
- Depression storage – 1mm (impervious areas).

Storm durations of 5 min, 10 min, 15 min, 20 min, 30 min, 1 hr, 2 hr, 3 hr, 4.5 hr and 6 hr were assessed for storms ranging from 1yr ARI to 100yr ARI events to determine existing drainage capacity and determine critical flood prone locations.

#### 4.2 Catchment Parameters

Catchment fraction impervious values has been determined based on the landtype breakup as shown in Figure 1 Site Landzone Map (Source: NSW Planning Portal) and applying the fraction impervious values provided in Table 3. Where a catchment includes multiple landtypes, a weighted average fraction impervious values has been adopted.

Both the existing and proposed models adopts the landtype breakup as shown in Figure 1 Site Landzone Map (Source: NSW Planning Portal).

*Table 3 Landtype % Impervious Values*

Landtype	Fraction Impervious (%)
R1 Residential	70
IN1 Industrial	70
RE1 Open Space	10
R5 Large Lot Residential	10

Catchment Time of concentration have been internally calculated by DRAINS using catchment parameter inputs for each catchment (flow path length, flow path slope and retardance coefficient).

#### 4.3 Tailwater Levels

Existing tailwater conditions for the outlet pipes has been investigated using available flood study reports. Two flood study reports were considered:

- Macquarie River (Narromine to Oxley Station) Floodplain Management Plan (2008).
- Narromine Riverbank Levee Feasibility Study, Lyall & Associates (2013)

Based on *Table 4.1 of the 'Macquarie River (Narromine to Oxley Station) Floodplain Management Plan'* (DECC, 2008), Macquarie River flood levels for the 5% AEP (20yr ARI) and 1% AEP (100yr ARI) has been estimated by applying Manning's Equation across the river cross section located at **Catchment C1** discharge location. The water level at this outlet has been calculated as 230.46m and 232.16m for the 5% and 1% AEP events, respectively.

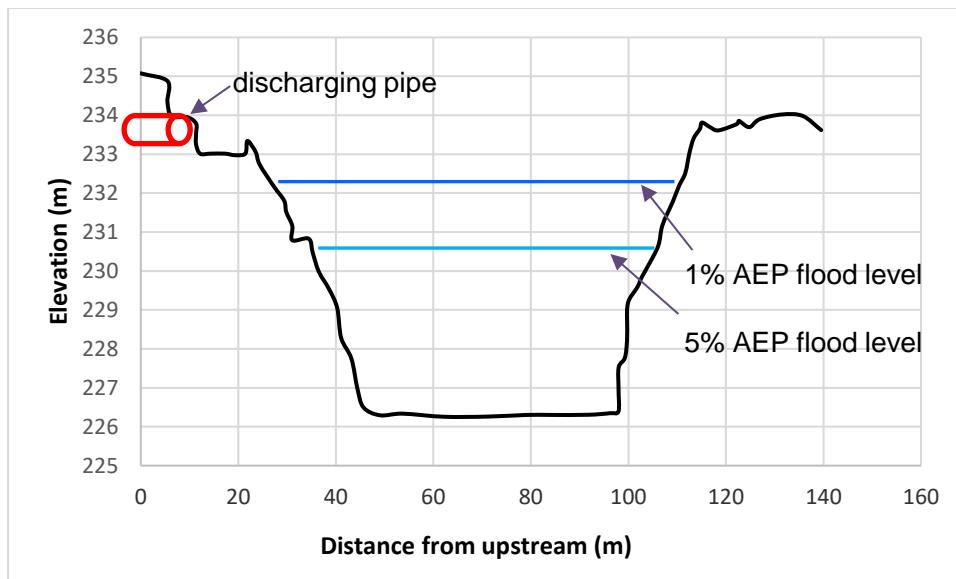


Figure 5 Estimated Macquarie River Flood Levels at Catchment C1 Outlet (based on 'Macquarie River (Narromine to Oxley Station) Floodplain Management Plan' (DECC, 2008)

The 'Narromine Riverbank Levee Feasibility Study' (Lyall & Associates, 2013) report also provides a peak 1%AEP Macquarie River flood level adjacent to the study area (approximately MIKE11 River Chainage 7.30). 5%AEP flood levels were not provided in the report. 1%AEP flood levels from the 'Narromine Riverbank Levee Feasibility Study' (Lyall & Associates, 2013) is presented in Figure 6.

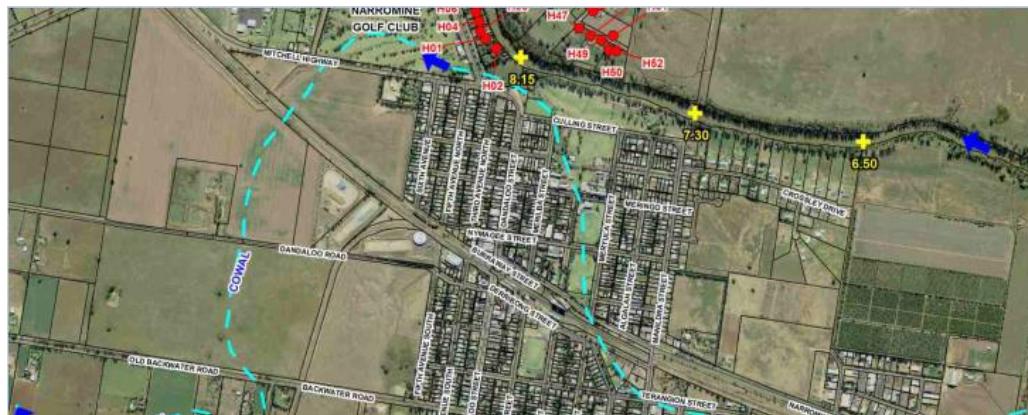


TABLE 3.2  
COMPARISON OF 1% AEP PEAK FLOOD LEVELS  
PRESENT STUDY VS LACE, 2009a

MIKE 11 Model Reach	MIKE 11 River Chainage	Location	Peak Flood Level (m AHD)		Difference in Peak Flood Levels <sup>(3)</sup> (Present vs 2009) (m)
			LACE, 2009a	Present Study	
Macquarie River <sup>(1)</sup>	0.00	Upstream limit of hydraulic model	242.50	243.2	+0.70
	1.35		242.37	243.04	+0.67
	3.25	Adjacent to eastern end of River Drive	241.93	242.24	+0.31
	5.50		240.66	240.81	+0.15
	6.50	Adjacent to eastern end of Crossley Drive	240.07	240.43	+0.36
	7.30	Adjacent to northern end of Manildra Street	239.55	239.98	+0.43
	8.15		239.08	239.55	+0.47
	8.75	Narromine-Eumungerie Road Bridge and Narromine Flood Gauge	238.67	239.12	+0.45
	9.40		238.15	238.74	+0.59

Figure 6 Catchment C1 Outlet Tailwater Levels (based on 'Narromine Riverbank Levee Feasibility Study' (Lyall & Associates, 2013))

Linearly interpolating the flood levels presented in Figure 6, '*Narromine Riverbank Levee Feasibility Study*' (Lyall & Associates, 2013) determined the 1%AEP flood levels at approximately 239.3 at the location of **Catchment C1** outlet location, drowning out the outlet pipe and flooding large areas of Narromine Township. It is noted that this 1%AEP flood level is also significantly higher than the flood level provided in '*Macquarie River (Narromine to Oxley Station) Floodplain Management Plan*' (DECC, 2008).

The '*Narromine Riverbank Levee Feasibility Study*' (Lyall & Associates, 2013) indicated that the Macquarie River overtops and floods majority of the town (100yr ARI flood level ~RL 239.40) in the 100yr ARI major event. As such, the proposed drainage design and analysis has been primarily focused on the peak 5yr ARI minor event.

Flood levels calculated from *Table 4.1* of the '*Macquarie River (Narromine to Oxley Station) Floodplain Management Plan*' (DECC, 2008) estimates the 20yr ARI (5%AEP) flood level to be well below the existing **Catchment C1** discharge outlet. As such, free discharge has been adopted for the 5yr ARI model runs.

## 5 Existing Model Methodology

This study extends to the entire Narromine township catchments as well including the previous DRAINS modelling of **Catchment C1** in '*Drainage Feasibility Study of the Narromine Northern Catchment (Rev A - dated 21/12/18)*' study. The previous model has been updated as necessary to include the external catchments (**Catchment Ext 1, Ext 2** and **Ext 3**) and updating the basin stage-storage data to reflect the updated existing surface data.

Modelling of the catchments has been undertaken using DRAINS (Ver 2020.061) incorporating the available drainage data and catchment analysis as discussed in Section 3.

DRAINS modelling has been limited to the main trunk lines for each of the catchments.

Open space areas at Narromine High School, Dundas Park and Payten were identified as providing flood storage before spilling onto the existing roads. Stage-storage of these areas were updated using LPI Lidar data and modelled as detention basin nodes in the DRAINS models.

Existing DRAINS model layout including existing flood storage areas are presented in Figure 7.

The Stage-Storage data for the existing storage areas (school oval basin, Payten Park basin, Dundas Park basin, Railway basin, Industrial Park basin) are presented in Appendix A.

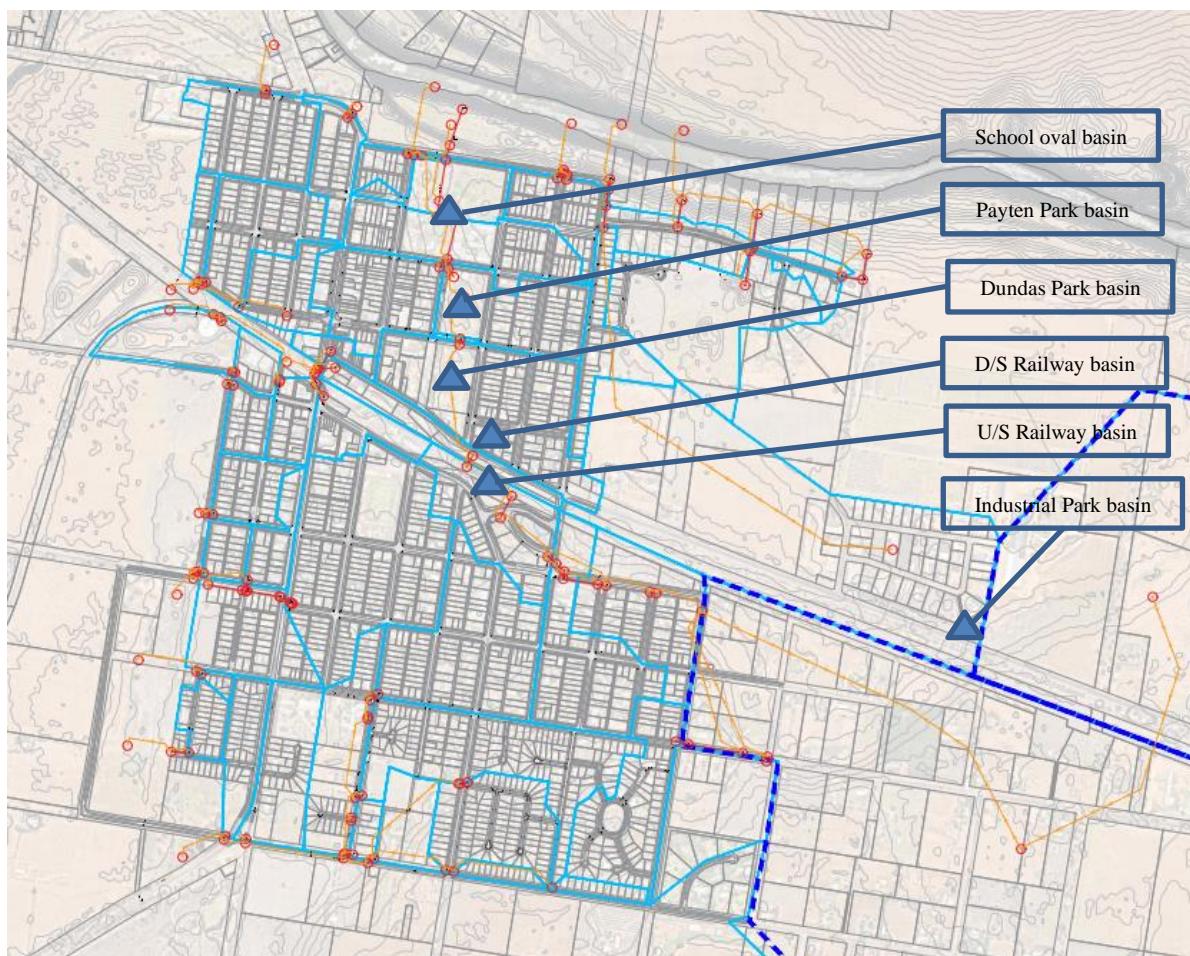


Figure 7 DRAINS Model Layout (Pit/Pipes in Red, Overland Flowpath Link in Orange)

## **6 Existing Model Results**

Many drainage design guidelines, such as AUSPEC, recommends the stormwater drainage to be designed to the major/minor drainage philosophy.

In the minor event (typically peak 5yr ARI), surface runoff are entirely collected and conveyed by the pit and pipe network and discharged to the downstream watercourse.

In the major event (typically peak 100yr ARI event), surface flows are conveyed via a combination of subsurface pit and pipe and overland flow within the road reserve. The pit and pipe network is generally sized to ensure overland flow is safely contained within the road reserve in the 100yr ARI event.

As discussed in Section 4.3 , Macquarie River overtops and floods majority of the town in the major event (100yr ARI flood level ~RL 239.40). As such, the existing DRAINS model has been run for 5yr ARI storms ranging from 5minutes to 6hours durations.

5yr ARI modelling results show the existing drainage within **Catchment C1, C2 and C8** to be significantly undersized with significant overland flow present. The modelling also indicates the drainage within the remaining catchments (**Catchment C3, C4a, C4b, C5, C6 and C7**) to be generally adequately sized with some drainage upgrades required for the peak 5yr ARI event.

Locations of undersized pipes is presented in

Figure 8.

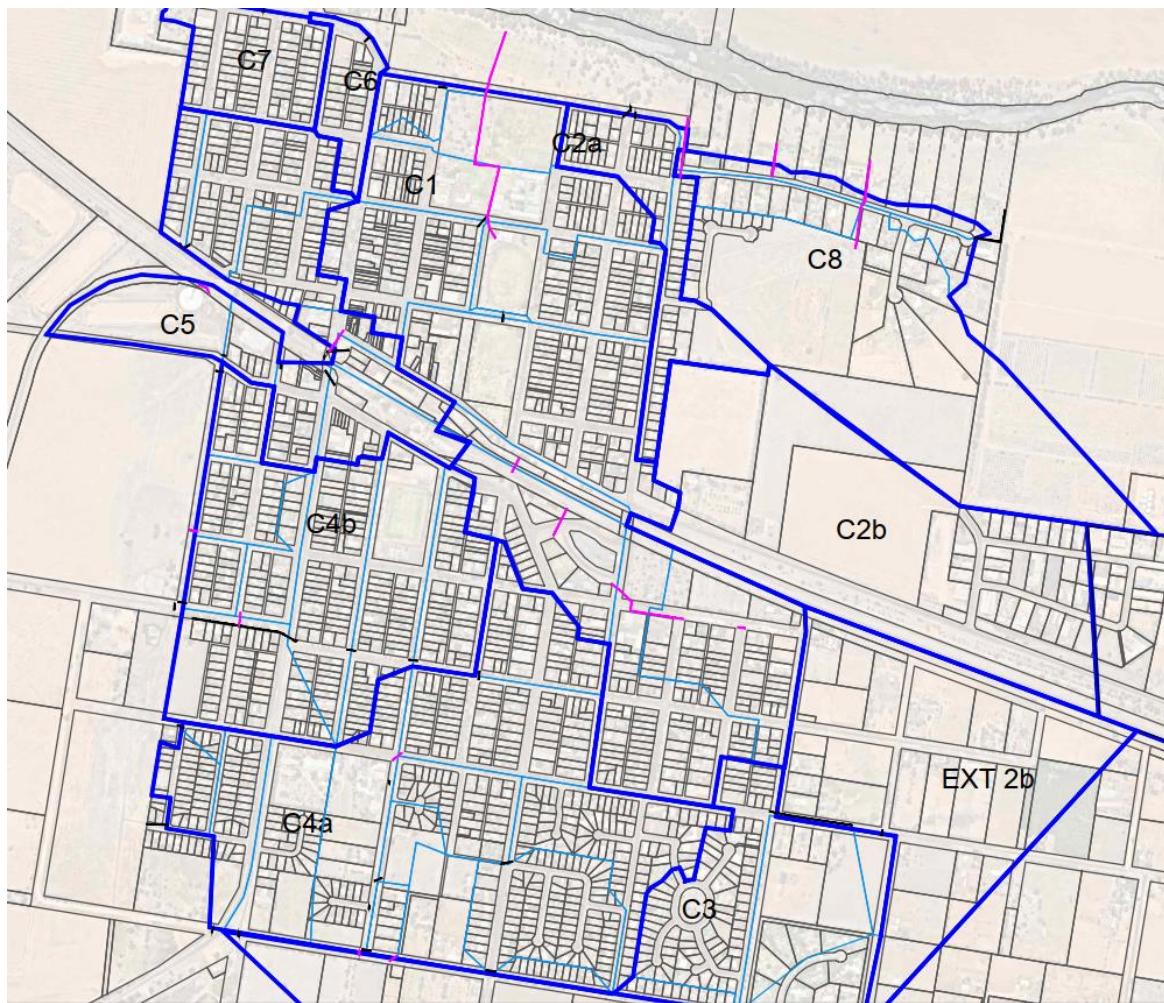


Figure 8 Existing Model – Location of under-capacity pipes in 5yr ARI event (Magenta)

#### 6.1.1 Catchment C1

Four key drainage areas are identified in Catchment C1:

- Terangion Street Drainage (including External Catchments (**Catchment Ext1, Ext2**) and **Catchment C3**)
- Narromine Railway Culvert Crossing (Int. Algalah St and Merulya St)
- Flood Storage Areas (Payten Park, Dundas Park, Narromine High School Oval)
- Macquarie River Discharge Point (Int. Culling St and Meryula St)

The upstream External Catchments, **Catchment Ext 1** and **Ext2** (refer Figure 4), has a total contributing watershed area of 495ha draining to **Catchment C1**. While the landuse of the external catchment is largely undeveloped/existing, the external catchment is approximately four times the size of the internal **Catchment C1** (125ha). The DRAINS modelling indicates the external catchments as having the most significant detrimental impact to the existing drainage network.

Unsurprisingly, the 5yr ARI run results show significant overland flooding within Terangion Street drainage (Upstream end of Catchment C1) as the existing drainage is inadequately sized to convey the external catchment runoff.

The modelling indicates that the single 1000x800 RCBC (IL 237.63) underneath the railway line to be undersized and overtops the existing **C1-Railway Embankment (SAG - RL 238.15)** in the 5yr ARI runs even with the inclusion of the natural detention basin (DRAINS Node U/S Railway) as presented in Figure 9.

A second natural detention basin (DRAINS Node D/S Railway) is formed by **C1 – Mitchell Hwy Embankment (SAG – RL 238.3)** and is located immediately downstream of the **C1-Railway Embankment (SAG – RL 238.15)**. The embankment is locally higher than the adjacent grassed corridor with no pipe crossing present to relieve the flood storage area. The basin continues to pond until overtopping the embankment sag RL 238.3 which subjects the railway culvert to undesirable tailwater conditions. The '**D/S Railway**' basin node provides little benefit to drainage in its vicinity. Combination of Mitchell Hwy embankment sag RL being 150mm higher than the Railway Embankment sag RL, undersized RCBC underneath the railway and large upstream flows results in the flooding issues presented in Figure 9.

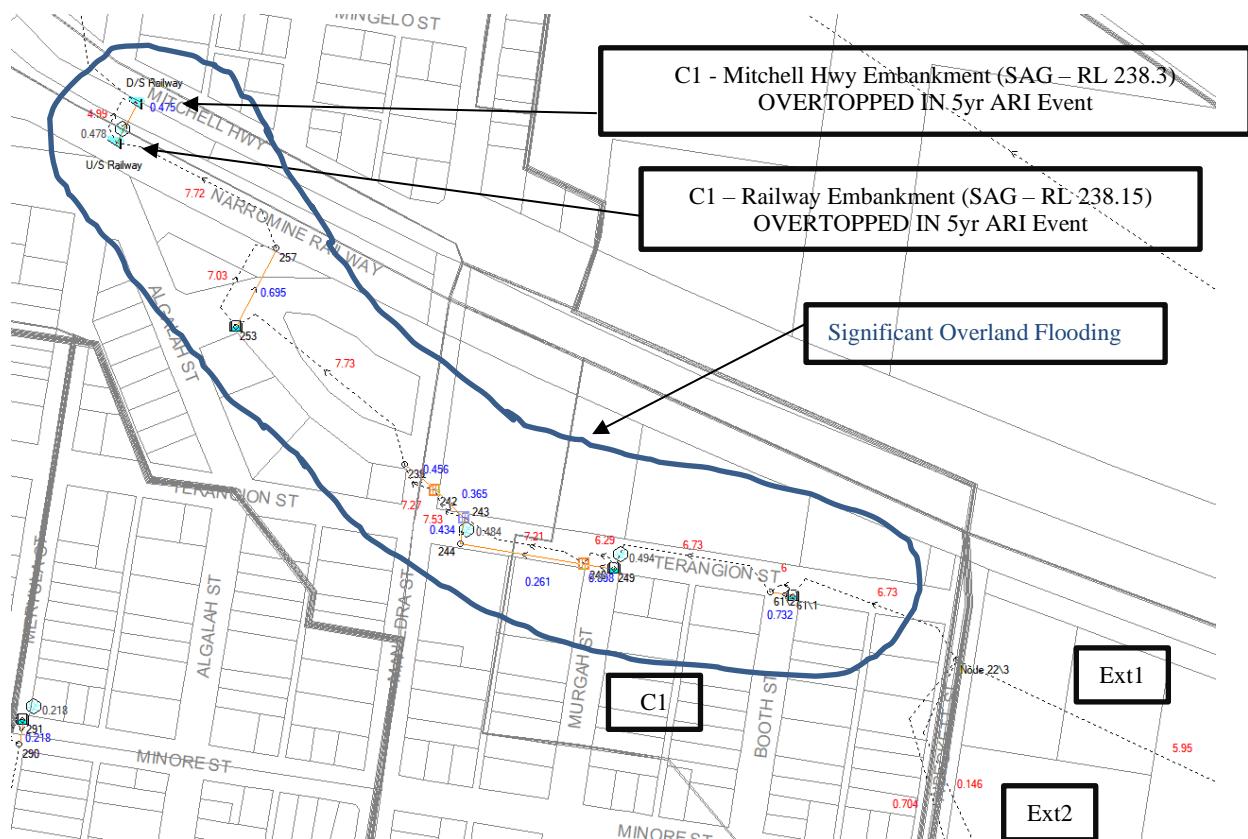


Figure 9 Existing DRAINS 5yr ARI Results (Catchment C1) – External Catchment Area/Railway Crossing  
Peak catchment flow rates in  $m^3/s$  are given black, pipe flows in blue and overflows in red

Further downstream to the flood storage areas (Payten Park, Dundas Park, Narromine HS), there are still significant overland flooding between the flood storage areas at Nymagee St, Meringo St and Culling St in the peak 5yr ARI event as shown in Figure 10.

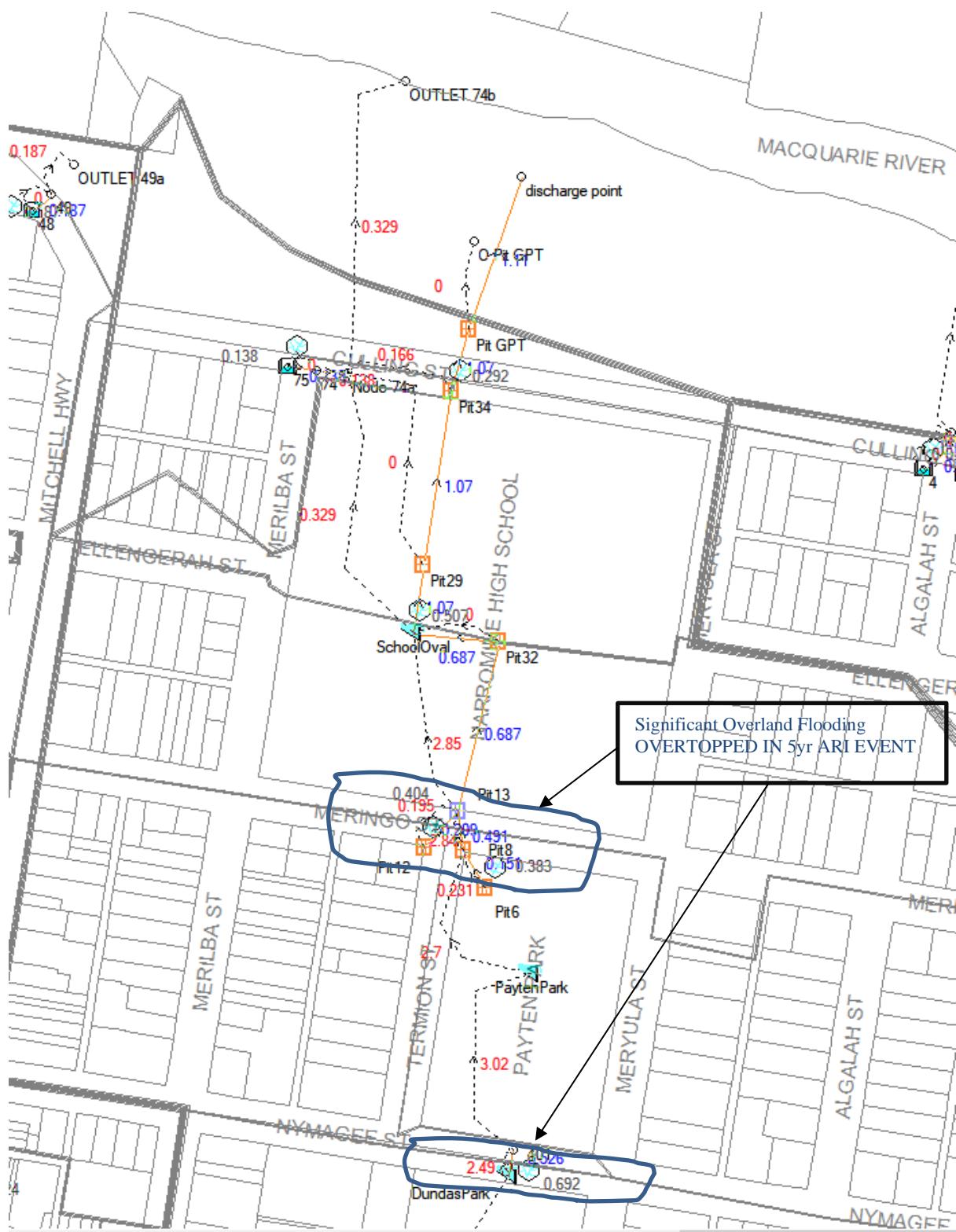


Figure 10 Existing DRAINS 5yr ARI Results (Catchment C1) – Flood Storage Areas/Macquarie River Discharge Location

Peak catchment flow rates in  $m^3/s$  are given **black**, pipe flows in **blue** and overflows in **red**

### 6.1.2 Catchment C2

Two key locations are identified in Catchment C2:

- Upstream Industrial/Large Lot Residential Catchment (**Catchment C2b**)
- Trunk drainage along Manildra St.

The existing lidar surface indicates the upstream industrial/large lot residential catchment, **Catchment C2b**, flows overland through the rear of existing lots at the intersection of Manildra and Nymagee St and onto the existing Manildra St pit and pipe network. Available pit and pipe data indicate that there are no interallotment drainage pits/pipes and easement to convey the overland flow from **Catchment 2b** to the existing drainage network.

Furthermore, the existing drainage along Manildra St has insufficient capacity for the 5yr ARI runoff generated from its upstream catchment. As such, the DRAINS modelling indicates significant overland flows along Manildra St in the peak 5yr ARI event as shown in Figure 11.

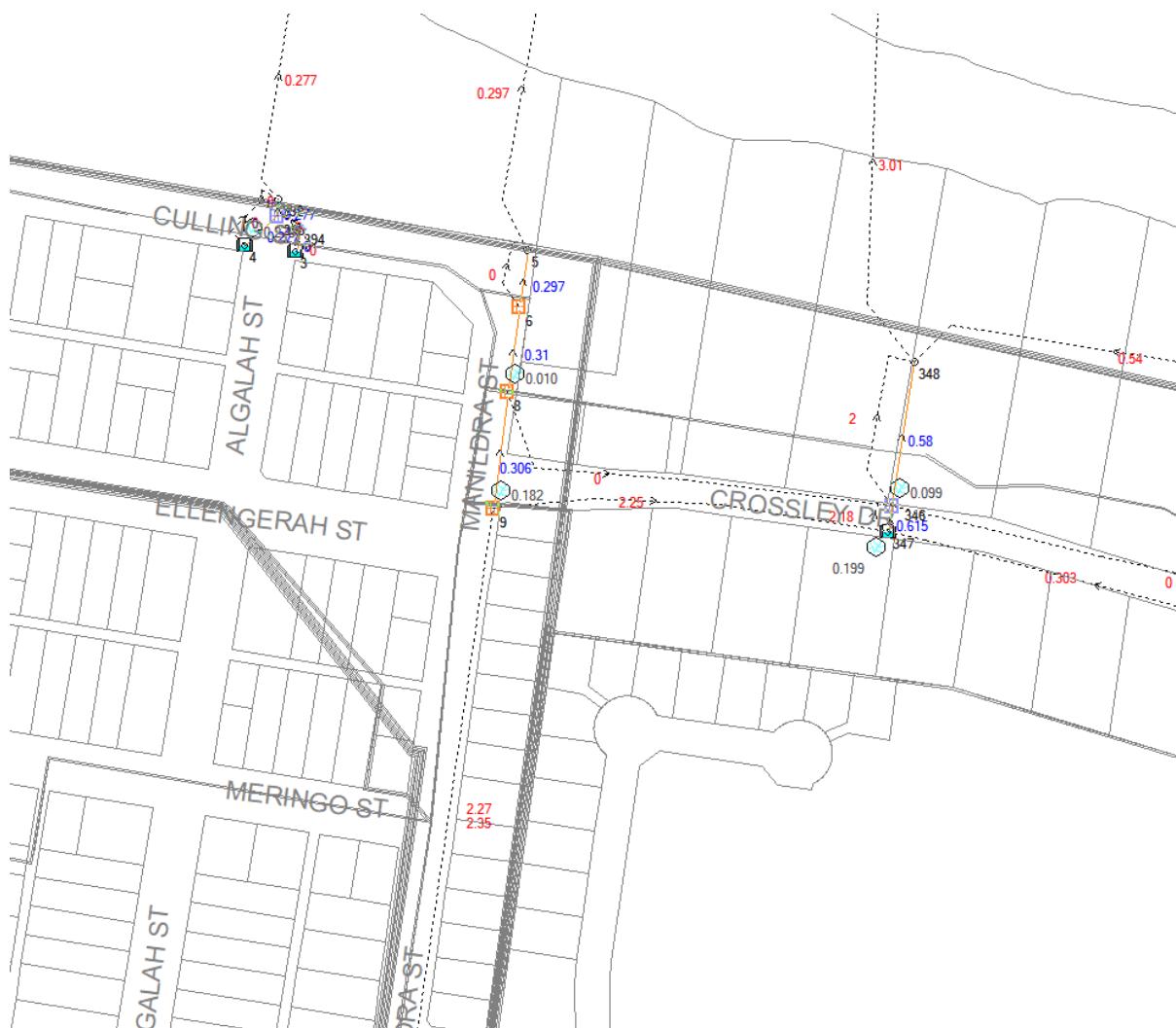


Figure 11 Existing DRAINS 5yr ARI Results (Catchment C2) – Trunk drainage along Manildra St with overland flow from upstream Catchment 2b

Peak catchment flow rates in  $\text{m}^3/\text{s}$  are given **black**, pipe flows in **blue** and overflows in **red**

### 6.1.3 Catchment C3

**Catchment C3**, of approximately 27.8ha of low-density residential housing, drains to the existing drainage and swale along Webbs Siding Rd and overland flows through existing property towards the Terangion Street drainage within **Catchment C1** as discussed in Section 6.1.1.

5yr DRAINS results indicate that the street drainage within **Catchment C3** is suitably sized to convey the peak 5yr ARI event.

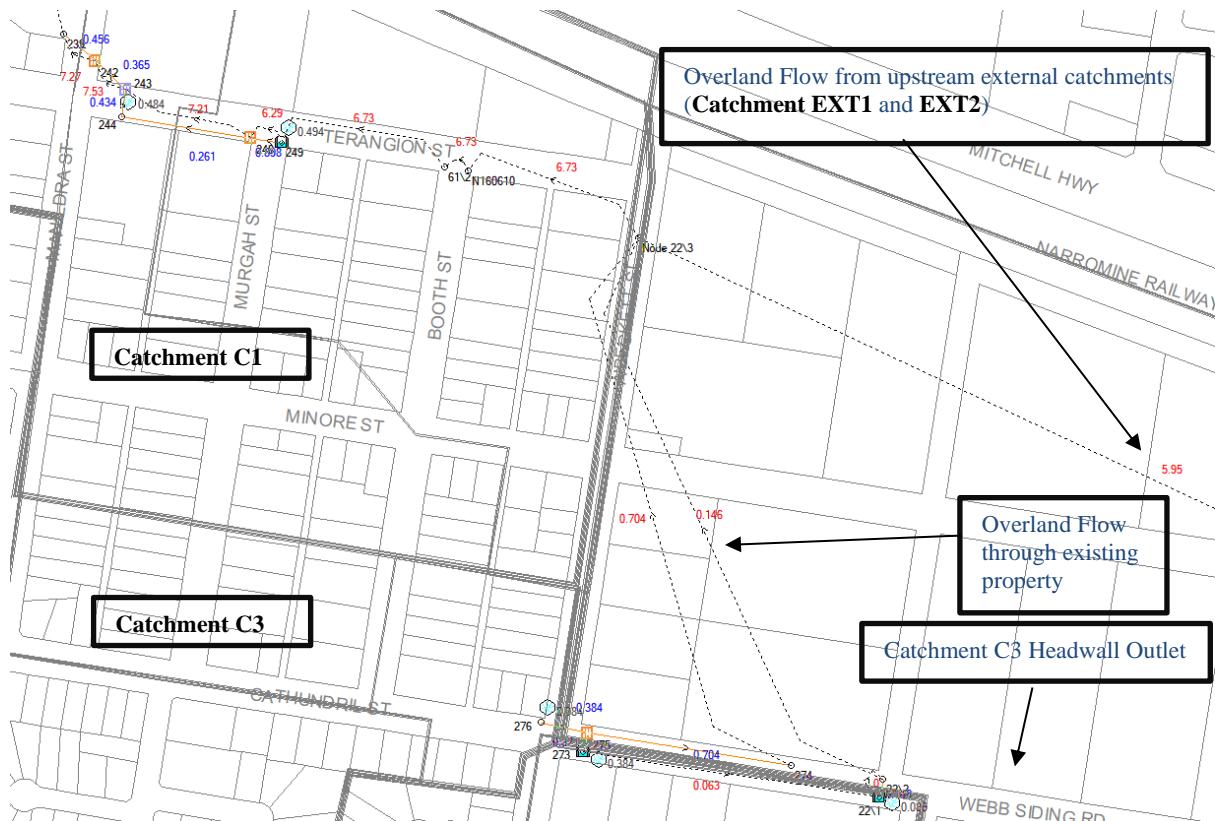


Figure 12 Existing DRAINS 5yr ARI Results (Catchment C3) – Piped and Swale Drainage along Webbs Siding Rd  
Peak catchment flow rates in  $m^3/s$  are given **black**, pipe flows in **blue** and overflows in **red**

### 6.1.4 Catchment C4

Three key locations are identified in Catchment C4:

- Drainage/Swale along Temoin St
- Trunk drainage/Swale along Nellie Vale Rd and Dappo Rd
- Drainage along Tancred St, Cathundril St, Backwater Rd and Terangion St

Catchments draining to Temoin St are conveyed towards the trunk drainage and swale along Nellie Vale Road via a combination of piped drainage running underneath existing roads/driveways and swales as shown in Figure 13. Some overland flows are present overtopping the pipe headwall inlet onto the existing road. The existing swale capacity along Temoin St is calculated to be approximately  $1.98m^3/s$  at 0.3% grade. Overland flows along Temoin are contained within the roadside swale without overtopping in the 5yr ARI event.

Similarly, the trunk drainage along Nellie Vale Road and Dappo Road are conveyed to the downstream discharge location via a combination of piped drainage under roads and roadside swales as shown in Figure 13. The existing swale capacity along Nellie Vale Rd and Dappo Rd is calculated to be approximately  $0.5\text{m}^3/\text{s}$  at 0.3% grade. The overland flows along Nellie Vale Rd and Dappo Rd exceeds the swale capacity and is likely to spill onto the road in the 5yr ARI event.

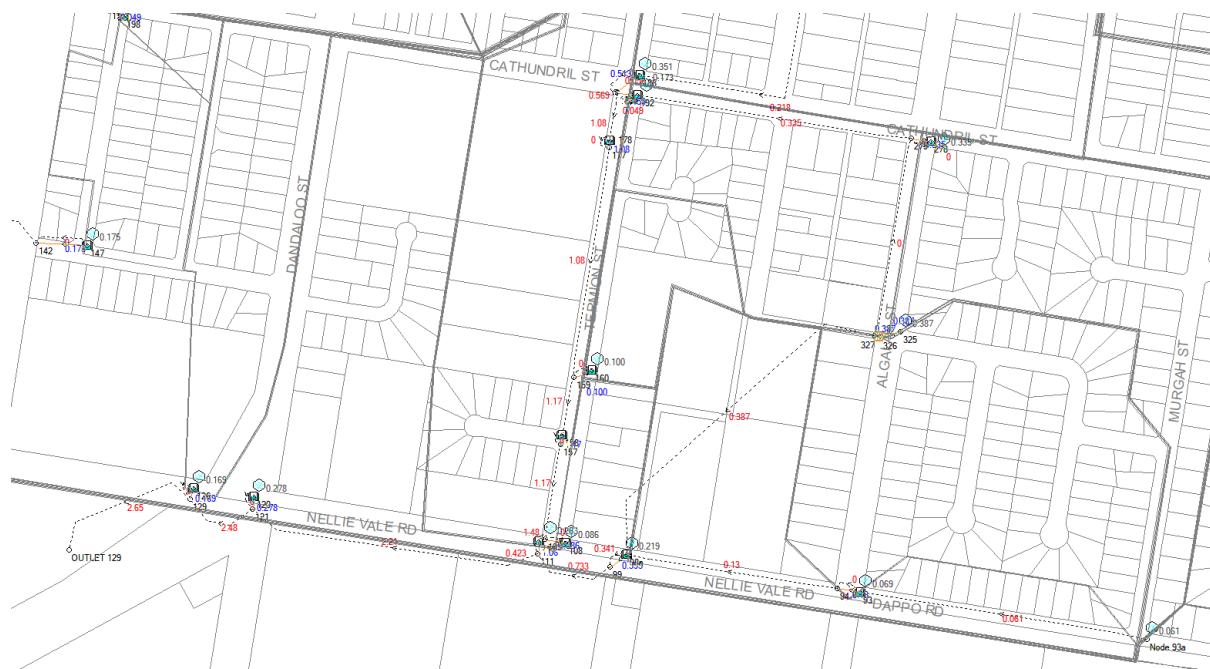


Figure 13 Existing DRAINS 5yr ARI Results (Catchment C4) – Trunk drainage along Temoin St, Nellie Vale Rd and Dappo Rd

Peak catchment flow rates in  $\text{m}^3/\text{s}$  are given **black**, pipe flows in **blue** and overflows in **red**

Catchments draining along Tancred St, Cathundril St, Backwater Rd and Terangion St are relatively small. The existing drainage along these roads are generally sufficiently sized to entirely convey 5yr ARI event flows within the pipes to be discharged westwards towards the existing open channel, ultimately draining to the south-western wetlands. Small overland flows are reported along Backwater Rd and Terangion St in the 5yr ARI event.

#### 6.1.5 Catchment C5

Catchment C5 generally drains westwards towards existing swales adjacent to the Narromine Railway Line via combination of piped and overland flow within the kerb and gutter as shown in Figure 14. The 5yr ARI results show that the existing drainage are sufficiently sized to convey the 5yr ARI flows, although overtopping flows have been reported in some areas. Swale information adjacent the railway line was not available to check existing swale capacity.

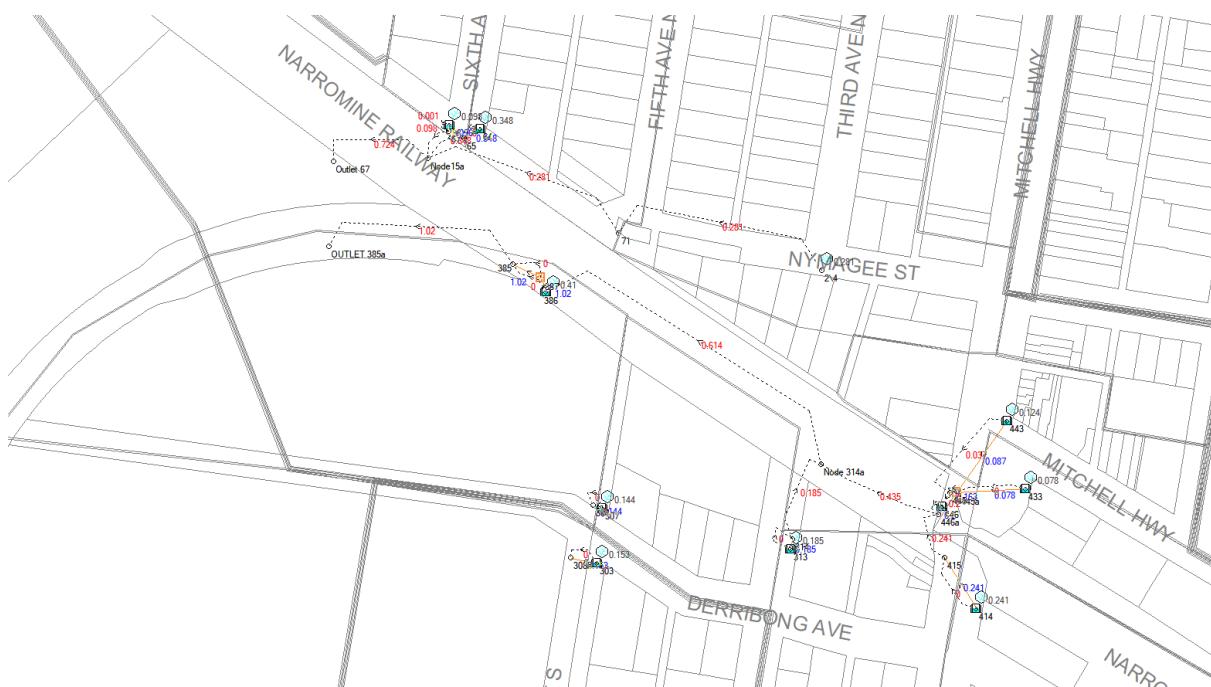


Figure 14 Existing DRAINS 5yr ARI Results (Catchment C5) – Drainage along Nymagee St, Dandaloo St and Derribong Ave

Peak catchment flow rates in  $m^3/s$  are given **black**, pipe flows in **blue** and overflows in **red**

### 6.1.6 Catchment C6 and C7

Catchment C6 and C7 are relatively small catchments of approximately 9.8ha and 4.8ha respectively as shown in Figure 15. No local road drainage are present within the catchments. Surface flows are collected via kerb and gutter to the trunk drainage pipe crossing Mitchell Hwy. The trunk drainage conveys the collected surface runoff and is discharged into Macquarie River.

The 5yr ARI results show that the existing trunk drainage are sufficiently sized to convey the 5yr ARI flows entirely within the pipe.

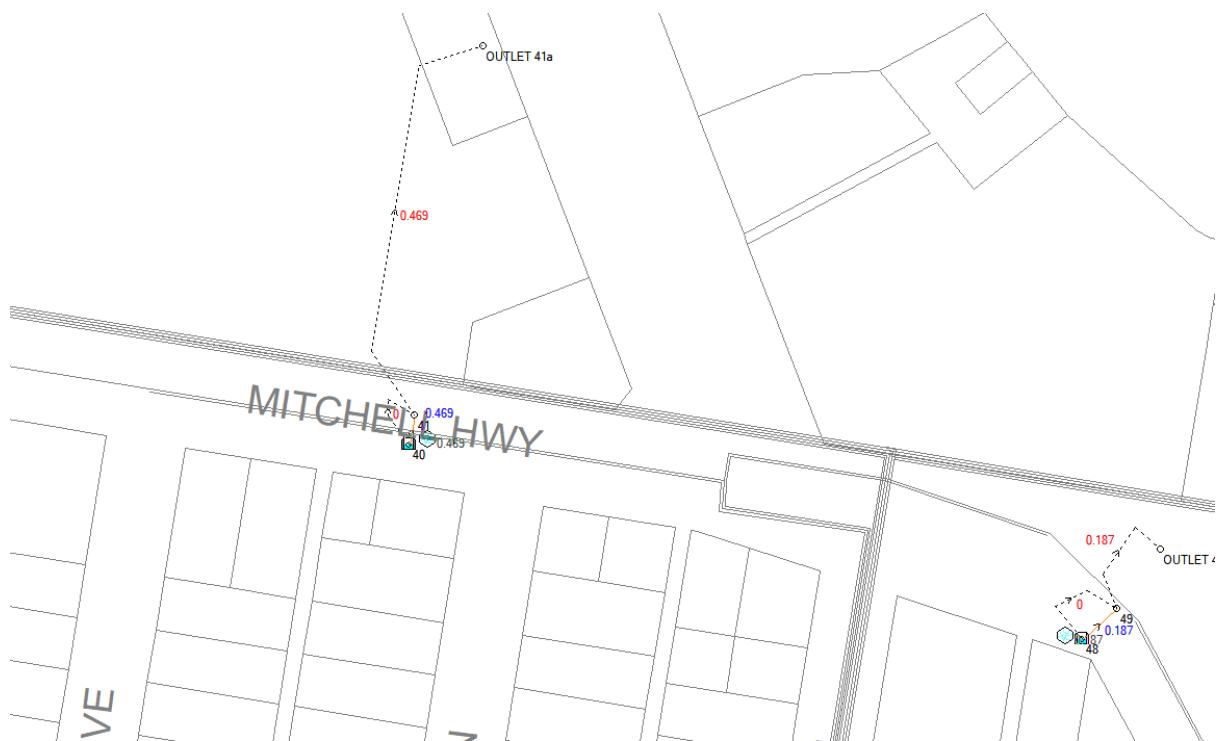


Figure 15 Existing DRAINS 5yr ARI Results (Catchment C6 and C7) –Drainage at intersection of Mitchell Hwy and Fifth Ave and Third Ave

Peak catchment flow rates in  $m^3/s$  are given **black**, pipe flows in **blue** and overflows in **red**

#### 6.1.7 Catchment C8

Three key locations are identified in Catchment C8:

- Trunk drainage running through 7 Crossley Dr
- Drainage running through 13 Crossley Dr
- Drainage running through 29 Crossley Dr

Catchment C8 generally drains northwards with an existing sag inlet pit located at 7 Crossley Drive. 5yr ARI results show that the existing drainage are undersized to convey catchment runoff with overland flow at most pit/pipe locations.

#### 6.1.8 Assumptions, Limitations and Other Items of Note

Following observations have been noted during review and analysis of the existing drainage network:

- Trapped low points have been identified at road sag locations without a sag pit
- DRAINS does not allow for bifurcation of overland flowpaths. Bifurcation of surface flows are likely at intersections and may not be accurately captured with DRAINS (2D hydrologic/hydraulic modelling software such as TUFLOW would be more appropriate).

### Attachment No. 3



Figure 16 Existing DRAINS 5yr ARI Results (Catchment C8) –Drainage at 7, 13 and 29 Crossley Drive  
Peak catchment flow rates in  $\text{m}^3/\text{s}$  are given **black**, pipe flows in **blue** and overflows in **red**

## 7 Proposed Drainage Strategy

Two proposed options have been developed to contain surface runoff within the pit and pipe network and surface ditch drains/open swales for the peak 5yr ARI event to alleviate existing flood prone areas in accordance with the minor drainage philosophy recommended in ARR87 and typically specified in AUSPEC.

As discussed in Section 4.3 , Macquarie River overtops and floods majority of the town in the major event (100yr ARI flood level ~RL 239.40). As such, the DRAINS modelling results focuses on the 5yr ARI minor event.

The two proposed options are:

- **Proposed Option 1**

The following drainage works are proposed in Option1 (Refer to Figure 17 for drainage works location):

1. The large external catchments, **Catchment Ext 1** and **Ext2**, currently draining through the town is a significant factor in the existing flooding issues as discussed in Section 3.3. It is proposed to divert the existing catchments away from the **Catchment C1** drainage network, providing significant relief to the existing drainage network. Based on the existing topography, approximately 92% of the external catchment (**Catchment Ext1 + Ext2a** - 460ha) will be diverted, via open channel, northwards towards Macquarie River. Remaining 8% of the external catchment (**Catchment Ext2b** - 40ha) will drain to **Catchment C1** as presented in Figure 17.
2. The large industrial **Catchment C2b** currently drains through the rear of properties and to the existing drainage network along Manildra Street located within **Catchment C2a**. The existing 750RCP line along Manildra Street does not have the capacity to accommodate runoff from **Catchment C2b**. It is proposed to divert **Catchment C2b** to **Catchment C8** away from Manildra Street via an open channel.
3. The existing drainage at Terangion Street has insufficient capacity to convey runoff from **Catchment C8** and **Ext2b** even with the diversion of the external catchments, A bypass channel, adjacent the Narromine Railway line, is proposed to divert **Catchment C8** and **Ext2b** away from the existing drainage along Terangion street and to the railway culvert adjacent Narromine Christian School.
4. As discussed in Section 6.1.1, the flood storage area (**Node D/S Railway**) immediately downstream of the railway culvert creates tailwater conditions reducing the capacity of the upstream railway culvert and overtopping of the railway embankment. A pipe crossing underneath Mitchell Highway is proposed to provide a hydraulic connection between the flood storage area and Dundas Park to prevent the overtopping of the railway or Mitchell Highway in the peak 5yr ARI event.

Existing flood storage areas at Narromine High School Oval, Payten Park and Dundas Park remains unchanged.

5. Catchment C4a drains via combination of open drains and pit and pipe towards Nellie Vale Road. An existing open drain collects and conveys **Catchment C4a** runoff to the existing wetlands appears to be undersized. An upgrade of the existing open drain is

proposed to convey **Catchment C4a** runoff in the 5yr ARI event without flooding Nellie Vale Road.

6. Some pipe sections have insufficient capacity even with the proposed drainage works discussed above. Drainage upgrades are required to contain runoff within the 5yr ARI pit and pipe network in the 5yr ARI event.

Pipe capacity has been increased by providing multiple pipes of equal size and invert level to the existing pipe. Where pit capacity is an issue, a ‘dummy pit’ with large inlet capacity has been provided to ensure surface runoff is captured and conveyed by the subsurface drainage system.

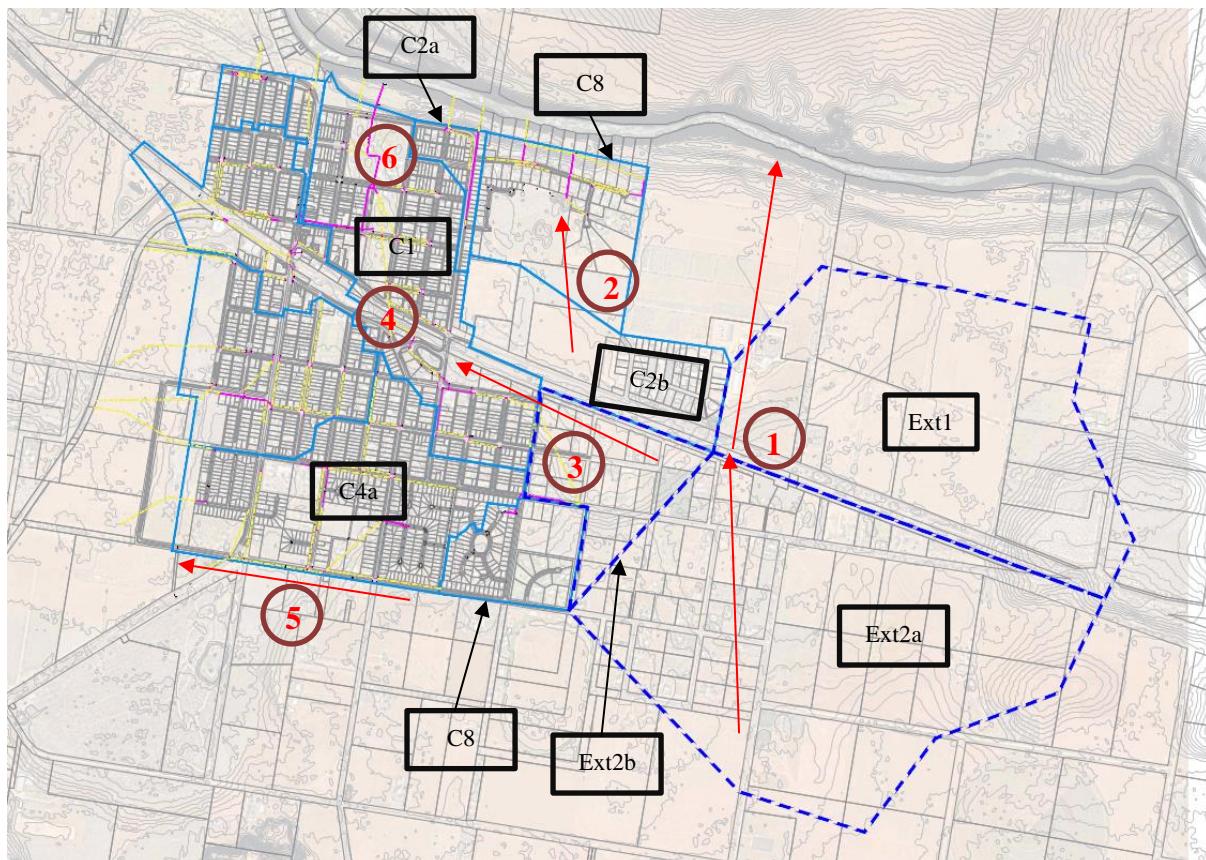


Figure 17 Proposed Option 1 – Proposed Drainage Works

- **Proposed Option 2**

Option 1 requires significant upsizing of existing drainage to manage flooding in the 5yr ARI event. The works are anticipated to include excavation, removal/relaying of pipes and road restoration works which may be overly cost prohibitive.

Option 2 proposes detention basins to reduce the scope and drainage upgrade works while providing green space, providing benefits to surrounding communities as well as achieving stormwater objectives as discussed in Option 1.

Two proposed detention basins are proposed and are located upstream of **Catchment C1** and **Catchment C2b** drainage network, outside of the developed residential areas. The basins will be sufficiently sized to attenuate upstream catchment flows and relieve downstream drainage network in **Catchment C1** and **Catchment C2a** as presented in Figure 18.

Provision of '*Proposed\_Basin01*' will remove the need to upgrade the 525RCP trunk line in **Catchment C8** proposed in Option1.

Provision of '*Proposed\_Basin02*' provides significant benefits to pipe network downstream of the basin in **Catchment C1**, reducing the scope of drainage upgrades required relative to Option1.

Option 2 may be a viable alternative if the costs of Option 1 is significantly higher than the cost to implement Option 2. The opinion of probable cost for both Option 1 and Option 2 is presented in Section 9.

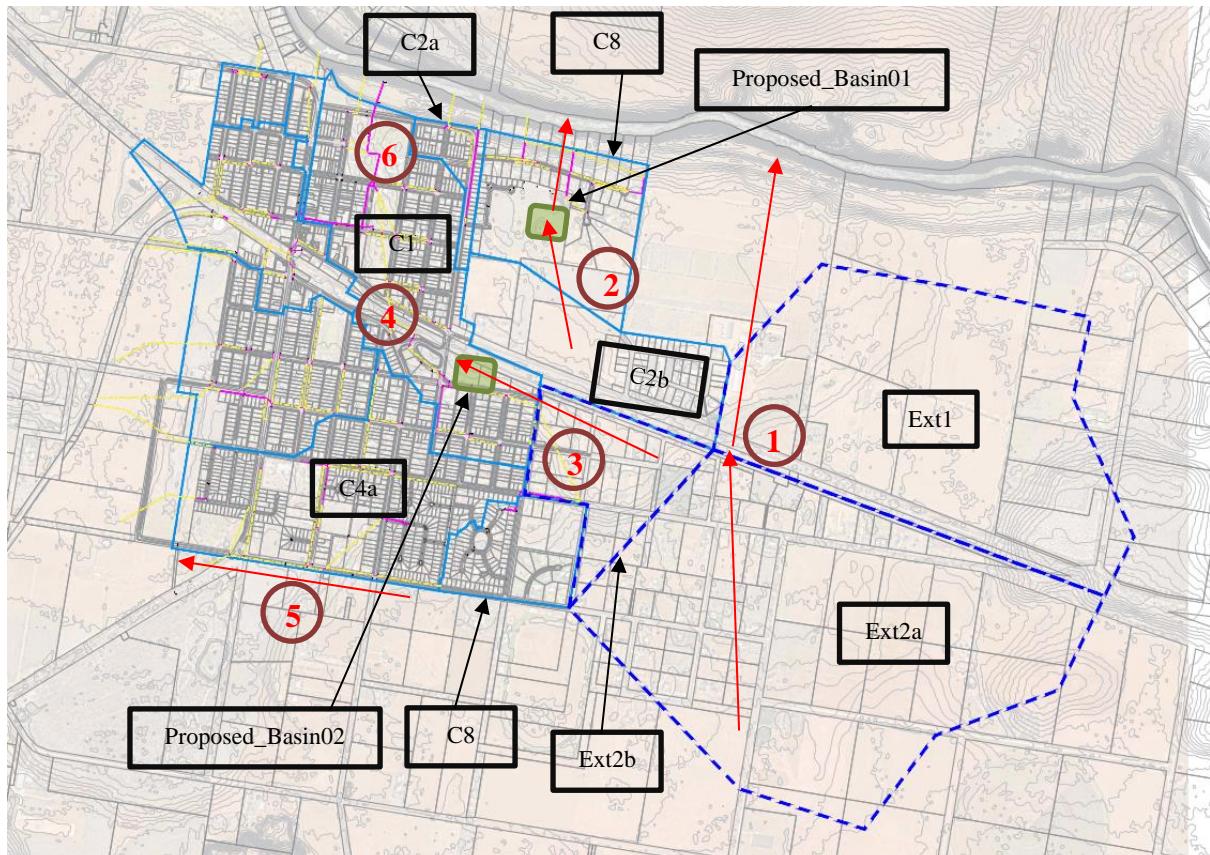


Figure 18 Proposed Option 2 - External Catchment (blue) with diverted overland path (red) and proposed detention basin locations (green)

## 8 Proposed Strategy Results

Modelling of the two drainage strategy, Option 1 and Option 2 has been undertaken using DRAINS (Ver 2020.061) by modifying the existing DRAINS model to provide the desired drainage outcomes

DRAINS modelling has been limited to the main trunk lines for each of the catchments.

### 8.1 Proposed Option 1 Model Results

Option 1 pipe upgrade locations are presented in Figure 19.

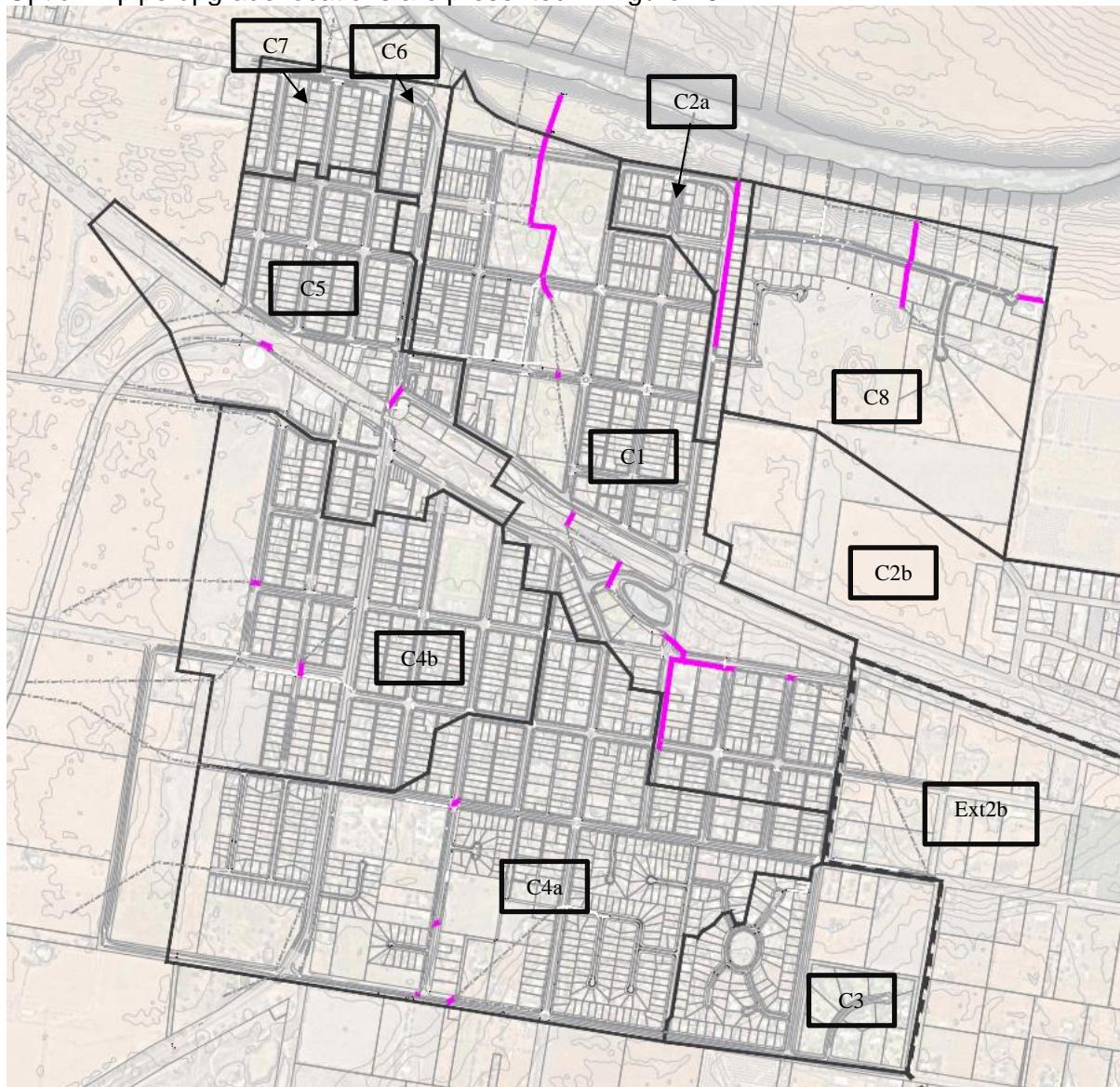


Figure 19 Proposed Option 1 - Pipe Upgrade Locations (Magenta)

#### 8.1.1 Catchment C1

The existing model reported significant flooding within **Catchment C1**. Even with the diversion of the external catchment away from **Catchment C1**, the existing drainage capacity is severely limited by the shallow pipe grades (most pipe grades range from 0.3% – 0.1%) and shallow pipe depths (majority pipes >0.5m cover). As such considerable drainage upgrades are required within **Catchment C1** to contain peak 5yr ARI flows within the drainage network.

As discussed in Section 6.1.1, the backwater effects from ‘**D/S Railway**’ node significantly reduces the capacity of the railway culvert crossing (**P 371**). As such, it is proposed to remove the ‘**D/S Railway**’ flood storage area and provide a new pipe (**Pipe113448**) to convey flows from railway culvert to Dundas Park flood storage area.

Table 4 outlines the proposed Option 1 pipe upgrades required and comparison with existing pipe sizes.

*Table 4 Proposed Works Option 1 – Catchment C1  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)*

<b>CATCHMENT C1 WORKS</b>			
<b>Pipe/OLF ID</b>	<b>Existing</b>	<b>Proposed - Option 1</b>	<b>Remarks</b>
<b>P 249</b>	1.2 x 0.3 RCBC	(3x) 1.2 x 0.3 RCBC	
<b>P 248</b>	1.5 x 0.3 RCBC	(3x) 1.2 x 0.3 RCBC	
<b>P 244</b>	1.2 x 0.3 RCBC	(3x) 1.2 x 0.3 RCBC	
<b>P 243</b>	1.2 x 0.3 RCBC	(3x) 1.2 x 0.3 RCBC	
<b>P 242</b>	1.2 x 0.3 RCBC	(3x) 1.2 x 0.3 RCBC	
<b>P 253</b>	1.2 x 0.3 RCBC	(3x) 1.2 x 0.3 RCBC	
<b>P 371</b>	750 RCP	(2X) 2.15x0.65 RCBC	Remove 'D/S Railway' flood storage area
<b>P 400</b>	1.2 x 0.2 RCBC	(4x) 1.2 x 0.25 RCBC	
<b>P Pit8</b>	0.9 x 0.6 RCBC	(2x) 0.9 x 0.6 RCBC	Pit 12, Pit6, Pit8 undersized
<b>P Pit13</b>	0.9 x 0.6 RCBC	(2x) 0.9 x 0.6 RCBC	
<b>P Pit32</b>	0.9 x 0.6 RCBC	(2x) 0.9 x 0.6 RCBC	
<b>P Basin-pit33</b>	0.9 x 0.6 RCBC	(2x) 0.9 x 0.6 RCBC	
<b>P Pit29</b>	0.9 x 0.6 RCBC	(2x) 0.9 x 0.6 RCBC	
<b>P Pit34</b>	(1x) 825	(2x) 825	
<b>P Pit GPT</b>	(2x) 600	(2x) 825	
<b>P Pit34</b>	(1x) 825	(2x) 825	
<b>Pipe113448</b>		(4x) 1.5 x 0.3 RCBC	Lay new pipe under Mitchell Hwy
<b>F Node 22\3</b>		5m x 0.4 SWALE	

### 8.1.2 Catchment C2

The existing model reports under capacity of the existing **Catchment C2a** drainage and significant overland flows. This is largely due to the huge upstream inflows from **Catchment C2b** draining through the rear of properties located at intersection of Manildra St and Nymagee St.

It is proposed to divert **Catchment C2b** away from the existing **Catchment C2a** pipe network and residential houses along Manildra avenue. 5yr ARI DRAINS modelling of this strategy indicates that the existing **Catchment C2a** pipe network requires no upgrade if **Catchment C2b** is diverted towards **Catchment C8**.

Table 5 outlines the proposed Option 1 upgrades required and comparison with existing pipe sizes.

*Table 5 Proposed Works Option 1 – Catchment C2  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)*

### **CATCHMENT C2 WORKS**

Pipe/OLF ID	Existing	Proposed - Option 1	Remarks
OF137218	N/A	10m x 0.4 SWALE	Construct swale to <b>Node 336</b> <b>(Catchment C8)</b>

#### 8.1.3 Catchment C3

Existing model reported some overland flows from **Node 273**. Twin 450 RCP has been provided to remove overland flows in Option 1.

Table 6 outlines the proposed Option 1 pipe upgrades required and comparison with existing pipe sizes.

*Table 6 Proposed Works Option 1 – Catchment C3*  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)

CATCHMENT C3 WORKS			
Pipe ID	Existing	Proposed - Option 1	Remarks
P 273	450 RCP	(2x) 450 RCP	

#### 8.1.4 Catchment C4

Runoff from **Catchment C4** is conveyed to the downstream discharge location via a combination of roadside open swales with piped headwall connections under roads and driveways. The existing model reports overflows in some locations due to undersized pipes.

Table 7 outlines the proposed Option 1 pipe upgrades required and comparison with existing pipe sizes.

*Table 7 Proposed Works Option 1 – Catchment C4*  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)

CATCHMENT C4 WORKS			
Pipe ID	Existing	Proposed - Option 1	Remarks
P 188	(1x) 1.2W x 0.3H	(1x) 1.5W x 0.3H	
P 110	(1x) 1.5W x 0.4H	(2x) 1.5W x 0.4H	
P 215	(1x) 0.4W x 0.2H	(3x) 0.4W x 0.2H	
P 98a	(1x) 1W x 0.25H	(2x) 1.2W x 0.25H	
P 188	(1x) 1.2W x 0.3H	(1x) 1.5W x 0.3H	
F 121		10m x 0.5 SWALE	Upgrade existing swale

#### 8.1.5 Catchment C5

Existing model reported some overland flows from **Catchment C5**.

Table 8 outlines the proposed Option 1 pipe upgrades required and comparison with existing pipe sizes.

*Table 8 Proposed Works Option 1 – Catchment C5  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)*

<b>CATCHMENT C5 WORKS</b>			
Pipe ID	Existing	Proposed - Option 1	Remarks
P 443	300 RCP	(2x) 300 RCP	
P 387	(2x) 600 RCP	(3x) 600 RCP	

#### 8.1.6 Catchment C6 and C7

5yr ARI flows are conveyed within the **Catchment C6** and **C7** drainage network. No works are proposed.

#### 8.1.7 Catchment C8

Runoff from **Catchment C8** is conveyed to the downstream discharge location via a combination of roadside open swales with piped headwall connections under roads and driveways. The existing model reports overflows in some locations due to undersized pipes and a large rural catchment draining to Node **336**.

Table 9 outlines the proposed Option 1 pipe upgrades required and comparison with existing pipe sizes.

*Table 9 Proposed Works Option 1 – Catchment C8  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)*

<b>CATCHMENT C8 WORKS</b>			
Pipe ID	Existing	Proposed - Option 1	Remarks
P 336	525 RCP	(3x) 900 RCP	Any overflow will spill to existing properties
P 337	525 RCP	(3x) 900 RCP	
P 338	525 RCP	(3x) 900 RCP	

#### 8.1.8 External Catchments (**Catchment Ext1** and **Ext2**)

The external **Catchment Ext1** and **Ext2**, contribute significant watershed area currently draining through the main township of Narromine. The 5yr DRAINS modelling indicate approximately 6m<sup>3</sup>/s peak combined flow generated by **Catchment Ext1** and **Ext2**, which has been modelled as rural landtype.

It is proposed to divert, via an open swale, the external catchment runoff towards Macquarie River northwards.

*Table 10 Proposed Works Option 1 – Catchment External  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)*

<b>CATCHMENT EXT WORKS</b>			
Pipe/OLF ID	Existing	Proposed - Option 1	Remarks
OF82144	N/A	15m x 0.4 SWALE	

## 8.2 Proposed Option 2 Model Results

Option 2 proposes two detention basins located upstream of **Catchment C1** and **C8** as well as the redirection of the industrial catchment (**Catchment C2b**) towards **Catchment C8** detention basin as presented in Figure 18.

Table 11 provides detention basin sizing details.

The Stage-Storage data for the proposed basins (Proposed\_Basin01, Proposed\_Basin02) are presented in Appendix A.

*Table 11 Detention Basin Sizing Details*

Proposed_Basin01	
<b>Base RL</b>	237.24
<b>Embankment RL</b>	238.5
<b>Base Area</b>	15000 m <sup>2</sup>
<b>Discharge Control</b>	525 RCP @ RL 237.24
<b>5yr ARI Live Storage</b>	19917 m <sup>3</sup>

Proposed_Basin02	
<b>Base RL</b>	237.6
<b>Embankment RL</b>	238
<b>Base Area</b>	30000 m <sup>2</sup>
<b>Discharge Control</b>	(2x)1.8x0.3 RCBC @ RL 237.6
<b>5yr ARI Live Storage</b>	11543 m <sup>3</sup>

Proposed Option 2 model resulted in reduction in pipe upgrades within **Catchment C1** and **C2**, compared to Option 1 pipe upgrades.

The proposed basins benefits downstream drainage network within **Catchment C1** and **C2** only. **Catchment C3** to **C8** will require drainage upgrades as proposed in Option 1.

Table 12 provides the Option 2 pipe sizing for **Catchment C1** and **C2**. Existing and Option 1 pipe sizing has also been provided for comparison.

*Table 12 Proposed Option 2 – Catchment C1 - Required Pipe Upgrades with Existing and Option 1 comparison  
(Ref: 055-20 Narromine\_PROPOSED\_REV00d\_upgrade pipes for 5yr ari.drn)*

CATCHMENT C1 PIPE UPGRADES				
Pipe ID	Existing	Proposed - Option 1	Proposed - Option 2	Remarks
<b>P 249</b>	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	
<b>P 248</b>	(1x) 1.5W x 0.3H	(3x) 1.5W x 0.3H	(3x) 1.5W x 0.3H	
<b>P 244</b>	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	
<b>P 243</b>	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	
<b>P 242</b>	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	
<b>P 253</b>	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	
<b>P 371</b>	(1x) 750	(2x) 2.15W x 0.65H	(1x) 2.15W x 0.65H	Remove 'D/S Railway' flood storage area

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<b>P 400</b>	(1x) 1.2W x 0.2H	(4x) 1.2W x 0.2H	(4x) 1.2W x 0.2H	
<b>P Pit8</b>	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	EXISTING	Pit 12, Pit6, Pit8 undersized
<b>P Pit13</b>	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	EXISTING	
<b>P Pit32</b>	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	EXISTING	
<b>P Basin-pit33</b>	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	EXISTING	
<b>P Pit29</b>	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	EXISTING	
<b>P Pit34</b>	(1x) 825	(2x) 825	EXISTING	
<b>P Pit GPT</b>	(2x) 600	(2x) 825	EXISTING	
<b>P 400</b>	(1x) 1.2W x 0.2H	(4x) 1.2W x 0.2H	(4x) 1.2W x 0.2H	
<b>Pipe113448</b>	-	(2x) 1.5W x 0.3H	2x) 1.5W x 0.3H	Lay new pipe under Mitchell Hwy
<b>F Node 22\3</b>		5m x 0.4 SWALE	5m x 0.4 SWALE	Lay new pipe under Mitchell Hwy

#### **CATCHMENT C8 PIPE UPGRADES**

Pipe ID	Existing	Proposed - Option 1	Proposed - Option 2	Remarks
<b>P 336</b>	(1x) 525	(3x) 900	EXISTING	
<b>P 337</b>	(1x) 525	(3x) 900	EXISTING	
<b>P 338</b>	(1x) 525	(3x) 900	EXISTING	

**9    Opinion of Probable Cost**

The proposed Option 1 Opinion of Probable Cost is shown in Table 13.

### Attachment No. 3



Table 13 Proposed Option 1 Opinion of Probable Cost

ID	Pipe/ OLF ID	LENGTH (m)	Existing	Proposed - Option 1	Option 1 Cost (\$)	Proposed - Option 2	Option 2 Cost (\$)
C1	P 249	26	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	\$ 63,370	(3x) 1.2W x 0.3H	\$ 63,370
C1	P 248	108	(1x) 1.5W x 0.3H	(3x) 1.5W x 0.3H	\$ 323,658	(3x) 1.5W x 0.3H	\$ 323,658
C1	P 244	23	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	\$ 54,312	(3x) 1.2W x 0.3H	\$ 54,312
C1	P 243	35	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	\$ 83,662	(3x) 1.2W x 0.3H	\$ 83,662
C1	P 242	34	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	\$ 80,602	(3x) 1.2W x 0.3H	\$ 80,602
C1	P 253	76	(1x) 1.2W x 0.3H	(3x) 1.2W x 0.3H	\$ 181,363	(3x) 1.2W x 0.3H	\$ 181,363
C1	P 371	38	(1x) 750	(2x) 2.15W x 0.65H	\$ 114,462	(1x) 2.15W x 0.65H	\$ 114,462
C1	P 400	18	(1x) 1.2W x 0.2H	(4x) 1.2W x 0.2H	\$ 60,185	(4x) 1.2W x 0.2H	\$ 60,185
C1	P Pit8	29	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	\$ 51,768	EXISTING	\$ -
C1	P Pit13	127	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	\$ 228,148	EXISTING	\$ -
C1	P Pit32	63	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	\$ 112,982	EXISTING	\$ -
C1	P Basin- pit33	50	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	\$ 89,471	EXISTING	\$ -
C1	P Pit29	127	(1x) 0.9W x 0.6H	(2x) 0.9W x 0.6H	\$ 229,468	EXISTING	\$ -
C1	P Pit34	47	(1x) 825	(2x) 825	\$ 37,529	EXISTING	\$ -
C1	P Pit GPT	117	(2x) 600	(2x) 825	\$ 93,905	EXISTING	\$ -
C1	Pipe1134 48	50	-	(2x) 1.5W x 0.3H	\$ 75,000	(2x) 1.5W x 0.3H	\$ 75,000
C2	P 9	67	(1x) 600	EXISTING	\$ -	EXISTING	\$ -
C2	P 8	50	(1x) 600	EXISTING	\$ -	EXISTING	\$ -
C2	P 6	33	(1x) 900	EXISTING	\$ -	EXISTING	\$ -
C3	P 273	15	(1x) 450	(2x) 450	\$ 5,355	(2x) 450	\$ 5,355
C4	P 188	30	(1x) 1.2W x 0.3H	(1x) 1.5W x 0.3H	\$ 45,453	(1x) 1.5W x 0.3H	\$ 45,453
C4	P 219	22	(1x) 0.6W x 0.22H	(2x) 0.6W x 0.22H	\$ 35,610	(2x) 0.6W x 0.22H	\$ 35,610
C4	P 110	13	(1x) 1.5W x 0.4H	(2x) 1.5W x 0.4H	\$ 20,138	(2x) 1.5W x 0.4H	\$ 20,138
C4	P 215	34	(1x) 0.4W x 0.2H	(3x) 0.4W x 0.2H	\$ 67,002	(3x) 0.4W x 0.2H	\$ 67,002
C4	P 98a	23	(1x) 1W x 0.25H	(2x) 1.2W x 0.25H	\$ 31,788	(2x) 1.2W x 0.25H	\$ 31,788

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<b>C5</b>	P 443	64	(1x) 300	(2x) 300	\$ 15,987	(2x) 300	\$ 15,987
<b>C5</b>	P 387	23	(2x) 600	(3x) 600	\$ 18,028	(3x) 600	\$ 18,028
<b>C8</b>	P 336	106	(1x) 525	(3x) 900	\$ 169,490	EXISTING	\$ -
<b>C8</b>	P 337	14	(1x) 525	(3x) 900	\$ 22,709	EXISTING	\$ -
<b>C8</b>	P 338	104	(1x) 525	(3x) 900	\$ 167,154	EXISTING	\$ -
<b>C1</b>	F Node 22\3	1530	-	5m x 0.4 SWALE	\$ 250,533	5m x 0.4 SWALE	\$ 250,533
<b>C2</b>	OF13721 8	1330	-	10m x 0.4 SWALE	\$ 217,801	10m x 0.4 SWALE	\$ 217,801
<b>C4</b>	F 121	680	-	10m x 0.5 SWALE	\$ 111,357	10m x 0.5 SWALE	\$ 111,357
<b>EXT</b>	OF82144	1230	-	15m x 0.4 SWALE	\$ 201,425	15m x 0.4 SWALE	\$ 201,425
<b>C8</b>	Proposed _Basin01	-	-	-	\$ -	15000 m <sup>2</sup> BASE	\$ 307,050
<b>C1</b>	Proposed _Basin02	-	-	-	\$ -	30000 m <sup>2</sup> BASE	\$ 614,100
<b>TOTAL</b>					\$ 3,259,732		\$ 2,978,259

**NOTE:**

- Land Acquisition cost rate of \$0.47/m<sup>2</sup> adopted
- Basin Construction cost rate of \$20/m<sup>2</sup> adopted
- Refer to '055-20 Cost Estimate.xlsx' for detailed cost estimate information
- Cost estimate figures are sensitive to land acquisition and basin construction cost rates

## 10 Conclusion

Storm Consulting has carried out an assessment of the existing stormwater network and prepared two options for upgrading this network to improve conditions of the catchment during the 5yr ARI rainfall events.

In conclusion, the DRAINS modelling identified several undersized existing drainage areas with overland flooding reported in the minor event (5yr ARI). Further, the current drainage system requires regular maintenance at culvert inlets/outlets to prevent the drainage system from getting blocked which compromises the drainage system efficiency.

The study has identified the following drainage works (Option 1) to improve the existing drainage network:

1. Diversion of External Catchments via open channel
2. Diversion of Industrial Catchment via open channel
3. Diversion of catchments upstream of Terangion Street drainage network
4. Remove flood storage area opposite Dundas Park and Narromine Railway
5. Upgrade existing catch drain at Nellie Vale Road
6. Upgrade existing drainage pit and pipes

In addition to the drainage works, the study has also identified an alternative option (Option 2) to utilise detention basins to reduce the scope of drainage upgrades required.

The existing and proposed DRAINS modelling and analysis have been limited to trunk drainage pits and pipes. It is recommended that further detailed modelling of smaller culvert crossings (e.g driveway culverts) be included in the detailed analysis to ensure adequate conveyance of flows into the trunk drainage network.

An opinion of probable cost has been provided to compare costs between Option 1 and Option 2. While Option 2 is relatively cheaper to Option 1, it is noted that the costing of Option 2 is highly sensitive to the land acquisition prices and basin construction costs.

## **Appendix A – Model Development**

*A1. DRAINS model data for the existing conditions*









# Attachment No. 3

OF149261 N184609 Node 22\3 29.2 10m wide | 0.25 0.25 0.4 0.3 0 24666834 1227.5

PIPE COVER DETAILS						
Name	Type	Dia (mm)	Safe Cover	Cover (m)		
P_66	RRU C2	300	0.1	-0.41	Unsafe	
P_2211	RRU C2	375	0.1	-0.41	Unsafe	
P_373	RRU C2	300	0.1	-0.33	Unsafe	
P_249	RCBC	0	0.15	-0.23	Unsafe	
P_248	RCBC	0	0.15	-0.28	Unsafe	
P_244	RCBC	0	0.15	-0.09	Unsafe	
P_243	RCBC	0	0.15	-0.13	Unsafe	
P_242	RCBC	0	0.15	0.06	Unsafe	
P_253	RCBC	0	0.15	-0.1	Unsafe	
P_158	RCBC	0	0.15	0.08	Unsafe	
P_433	RCBC	0	0.15	-0.06	Unsafe	
P_445a	RCBC	0	0.15	0.64		
P_443	RRU C2	300	0.1	-0.06	Unsafe	
P_120	RCBC	0	0.15	-0.13	Unsafe	
P_126	RRU C2	375	0.1	0.06	Unsafe	
P_147	RCBC	0	0.15	-0.06	Unsafe	
P_93	RRU C2	300	0.1	-0.12	Unsafe	
P_64	RCBC	0	0.15	-0.17	Unsafe	
P_188	RCBC	0	0.15	-0.33	Unsafe	
P_108	RCBC	0	0.15	-0.06	Unsafe	
P_198	RCBC	0	0.15	-0.12	Unsafe	
P_178	RRU C2	450	0.1	-0.71	Unsafe	
P_219	RCBC	0	0.15	-0.34	Unsafe	
P_40	RCBC	0	0.15	0.05	Unsafe	
P_75	RCBC	0	0.15	-0.09	Unsafe	
P_307	RCBC	0	0.15	-0.22	Unsafe	
P_341	RRU C2	525	0.1	-0.05	Unsafe	
P_342	RRU C2	525	0.1	-0.11	Unsafe	
P_336	RRU C2	525	0.1	0.03	Unsafe	
P_337	RRU C2	525	0.1	2.14		
P_338	RRU C2	525	0.1	-0.4	Unsafe	
P_4	RRU C2	450	0.1	0.21		
P_393	RRU C2	450	0.1	-0.26	Unsafe	
P_3	RRU C2	600	0.1	-0.48	Unsafe	
P_110	RCBC	0	0.15	-0.05	Unsafe	
P_386	RRU C2	600	0.1	0.39		
P_387	RRU C2	600	0.1	0.26		
P_376	RRU C2	375	0.1	-0.04	Unsafe	
P_215	RCBC	0	0.15	-0.12	Unsafe	
P_217a	RCBC	0	0.15	0.2		
P_214a	RCBC	0	0.15	0.15		
P_160	RCBC	0	0.15	-0.3	Unsafe	
P_98a	RCBC	0	0.15	0.06	Unsafe	
P_446	RRU C2	750	0.1	0.33		
P_303	RCBC	0	0.15	-0.13	Unsafe	
P_48	RRU C2	600	0.1	-0.37	Unsafe	
P_213	RCBC	0	0.15	-0.29	Unsafe	
P_414	RCBC	0	0.15	-0.3	Unsafe	
P_276	RCBC	0	0.15	0.28		
P_275	RRU C2	600	0.1	-0.32	Unsafe	
P_325	RCBC	0	0.15	0.18		
P_326	RCBC	0	0.15	0.03	Unsafe	
P_9	RRU C2	600	0.1	0.03	Unsafe	
P_8	RRU C2	600	0.1	0.4		
P_6	RRU C2	900	0.1	-1.23	Unsafe	
P_2112	RCBC	0	0.15	0.19		
P_2113	RCBC	0	0.15	0.34		
P_2112	RCBC	0	0.15	-0.7	Unsafe	
P_Basin-pit	RCBC	0	0.15	-0.7	Unsafe	
P_2129	RCBC	0	0.15	1.5		
P_2134	RRU C2	825	0.1	1.91		
P_Fit GPT	RRU C2	600	0.1	-0.49	Unsafe	
P_400	RCBC	0	0.15	-0.85	Unsafe	
P_116	RCBC	0	0.15	0.29		
P_118	RCBC	0	0.15	0.28		
P_371	RCBC	0	0.15	-1.23	Unsafe	
P_273	RRU C2	450	0.1	0.07	Unsafe	
P_347	RRU C2	525	0.1	-0.35	Unsafe	
P_346	RRU C2	525	0.1	-0.38	Unsafe	
P_291	RCBC	0	0.15	-0.3	Unsafe	
P_278	RCBC	0	0.15	-0.36	Unsafe	
P_192	RCBC	0	0.15	-0.24	Unsafe	
P_200	RCBC	0	0.15	-0.38	Unsafe	
P_203	RCBC	0	0.15	-0.15	Unsafe	
P_207	RCBC	0	0.15	-0.05	Unsafe	
P_208	RCBC	0	0.15	0.14	Unsafe	
P_218a	RCBC	0	0.15	0.13	Unsafe	
P_209a	RCBC	0	0.15	0.1	Unsafe	
P_213	RCBC	0	0.15	0.14	Unsafe	
P_216a	RCBC	0	0.15	0.12	Unsafe	
P_140	RCBC	0	0.15	0.03	Unsafe	
P_131	RRU C2	375	0.1	0.16		
Pipe10930	RCBC	0	0.15	0.2		

This model has no pipes with non-return valves

*A2. DRAINS model data for the proposed Option 1*









## Attachment No. 3

OF146085	131	130	1.8	238.07	10	1.7	8 m wide r	0.14	0.14	0.6	0.3	0	24366368	67.17
OF146067	130	140	3.6			8 m wide r	0.14	0.14	0.6	0.3	0		24366349	137.855
OF149269 HW47	N184609	N184609	0.6	240.3	50	1.7	Overflow a	0.05	0	0.6	0.3	0	24666847	14.814
OF205948 N184609	N101397		29.2			10m wide	0.25	0.25	0.4	0.3	0		37937058	1227.5

**PIPE COVER DETAILS**

Name	Type	Dia (mm)	Safe Cover	Cover (m)
P 66	RRJ C2	300	0.1	-0.41 Unsafe
P 221	RRJ C2	375	0.1	-0.41 Unsafe
P 373	RRJ C2	300	0.1	-0.33 Unsafe
P 249	RCBC	0	0.15	-0.23 Unsafe
P 248	RCBC	0	0.15	-0.28 Unsafe
P 244	RCBC	0	0.15	-0.09 Unsafe
P 243	RCBC	0	0.15	-0.13 Unsafe
P 242	RCBC	0	0.15	0.06 Unsafe
P 253	RCBC	0	0.15	-0.1 Unsafe
P 158	RCBC	0	0.15	0.08 Unsafe
P 433	RCBC	0	0.15	-0.06 Unsafe
P 445a	RCBC	0	0.15	0.64
P 443	RRJ C2	300	0.1	-0.06 Unsafe
P 120	RCBC	0	0.15	-0.13 Unsafe
P 126	RRJ C2	375	0.1	0.06 Unsafe
P 147	RCBC	0	0.15	-0.06 Unsafe
P 93	RRJ C2	300	0.1	-0.12 Unsafe
P 64	RCBC	0	0.15	-0.17 Unsafe
P 188	RCBC	0	0.15	-0.33 Unsafe
P 108	RCBC	0	0.15	-0.06 Unsafe
P 198	RCBC	0	0.15	-0.12 Unsafe
P 178	RRJ C2	450	0.1	-0.71 Unsafe
P 219	RCBC	0	0.15	-0.34 Unsafe
P 40	RCBC	0	0.15	0.05 Unsafe
P 75	RCBC	0	0.15	-0.09 Unsafe
P 307	RCBC	0	0.15	-0.22 Unsafe
P 341	RRJ C2	525	0.1	-0.05 Unsafe
P 342	RRJ C2	525	0.1	-0.11 Unsafe
P 336	RRJ C2	900	0.1	-0.38 Unsafe
P 337	RRJ C2	900	0.1	1.74
P 338	RRJ C2	900	0.1	-0.8 Unsafe
P 4	RRJ C2	450	0.1	0.21
P 393	RRJ C2	450	0.1	-0.26 Unsafe
P 3	RRJ C2	600	0.1	-0.48 Unsafe
P 110	RCBC	0	0.15	-0.05 Unsafe
P 386	RRJ C2	600	0.1	0.39
P 387	RRJ C2	600	0.1	0.26
P 376	RRJ C2	375	0.1	-0.04 Unsafe
P 215	RCBC	0	0.15	-0.12 Unsafe
P 217a	RCBC	0	0.15	0.2
P 214a	RCBC	0	0.15	0.15
P 160	RCBC	0	0.15	-0.3 Unsafe
P 08a	RCBC	0	0.15	0.06 Unsafe
P 446	RRJ C2	750	0.1	0.33
P 303	RCBC	0	0.15	-0.13 Unsafe
P 48	RRJ C2	600	0.1	-0.37 Unsafe
P 313	RCBC	0	0.15	-0.29 Unsafe
P 414	RCBC	0	0.15	-0.3 Unsafe
P 276	RCBC	0	0.15	0.28
P 275	RRJ C2	600	0.1	-0.32 Unsafe
P 325	RCBC	0	0.15	0.18
P 326	RCBC	0	0.15	0.03 Unsafe
P 9	RRJ C2	600	0.1	0.03 Unsafe
P 8	RRJ C2	600	0.1	0.4
P 6	RRJ C2	900	0.1	-1.23 Unsafe
P Pit12	RCBC	0	0.15	0.19
P Pit13	RCBC	0	0.15	0.34
P Pit32	RCBC	0	0.15	-0.7 Unsafe
P Basin-pit	RCBC	0	0.15	-0.7 Unsafe
P Pit29	RCBC	0	0.15	1.5
P Pit34	RRJ C2	825	0.1	1.91
P Pit GPT	RRJ C2	825	0.1	-0.73 Unsafe
P 400	RCBC	0	0.15	-0.85 Unsafe
P Pit6	RCBC	0	0.15	0.29
P Pit8	RCBC	0	0.15	0.28
P 371	RCBC	0	0.15	-0.98 Unsafe
P 273	RRJ C2	450	0.1	0.07 Unsafe
P 347	RRJ C2	525	0.1	-0.35 Unsafe
P 346	RRJ C2	525	0.1	-0.38 Unsafe
P 291	RCBC	0	0.15	-0.3 Unsafe
P 278	RCBC	0	0.15	-0.36 Unsafe
P 192	RCBC	0	0.15	-0.24 Unsafe
P 200	RCBC	0	0.15	-0.38 Unsafe
P 203	RCBC	0	0.15	-0.15 Unsafe
P 207	RCBC	0	0.15	-0.05 Unsafe
P 208	RCBC	0	0.15	0.14 Unsafe
P 218a	RCBC	0	0.15	0.13 Unsafe
P 209	RCBC	0	0.15	0.1 Unsafe
P 213	RCBC	0	0.15	0.14 Unsafe
P 216a	RCBC	0	0.15	0.12 Unsafe
Pipe11344	RCBC	0	0.15	-0.1 Unsafe
P 140	RCBC	0	0.15	0.03 Unsafe
P 131	RRJ C2	375	0.1	0.16
Pipe10920	RCBC	0	0.15	0.2

This model has no pipes with non-return valves

*A3. DRAINS model data for the proposed Option 2*











## **Appendix B - DRAINS Model Figures & Results**

*B1. DRAINS model results for the existing conditions under 5yr ARI design event*

## Attachment No. 3

DRAINS results prepared from Version 2020.061

PIT / NODE DETAILS Version 8						
Name	Max HGL	Max Pond	Max Surfac	Max Pond	Min	Overflow Constraint
	HGL	Flow Arriv	Volume	Freeboard	(cu.m/s)	(m)
66	238.17		0.098		0	0.001 Headwall height/system capacity
67	237.63		0.001			
22\1	239.36		0.146		0.07	0 None
22\2	238.7		0			
373	237.63		0.144		0.18	0 None
374	237.38		0			
249	239.01		7.221		-0.74	6.31 Headwall height/system capacity
248	238.22		6.31		0	7.231 Outlet System
244	238.26		0.484			
243	238.13	238.13	7.231	8.6	0	7.55 Outlet System
242	238.12		7.55		0.01	7.286 Inlet Capacity
239	237.84		7.286			
253	238.8		7.742		-0.79	7.046 Headwall height/system capacity
257	237.81		7.046			
158	238.05		1.174		0.25	0 None
157	237.81		0			
433	238.6		0.078		0.3	0 None
445a	238.29		0		0.8	0 None
445	238.21		0.037			
443	238.91		0.124		-0.02	0.037 Headwall height/system capacity
120	237.67		0.278		0.27	0 None
121	237.53		3.641			
126	237.67		0.169		0.16	0 None
129	237.58		3.914			
147	237.94		0.175		0.21	0 None
142	237.67		1.73			
93	241.21		0.131		0.32	0 None
94	240.59		0			
64	237.8		0.348		0.27	0 None
65	237.61		0			
188	238.43		0.569		-0.03	0.056 Headwall height/system capacity
183	238.15		0.056			
108	237.72		0.086		0.17	0 None
109	237.61		1.435			
198	238.18		0.049		0.28	0 None
197	238.09		1.71			
178	238.35		1.077		0.02	0 None
177	237.85		0			
219	237.86		0.229		-0.03	0.048 Headwall height/system capacity
901	237.32		0.048			
40	237.85		0.469		0.03	0 None
41	237.66		0			
75	238.02		0.138		0.12	0 None
74	237.75		0			
307	237.78		0.144		0.16	0 None
304	237.7		0			
341	238.78		0.122		0.32	0 None
342	238.59		0		0.9	None
343	238.13		0			
336	238.07		0.676		-0.15	0.6 Headwall height/system capacity
337	238.03		0.651		1.35	0.303 Inlet Capacity
338	237.61		0		1.84	0 None
345	236.31		0.122			
4	238.71		0.278		0.26	0 None
393	238.33	238.45	0	0	0.13	0 None
392	238		0			
3	238.62		0		0.75	0 None
394	238.41		0			
110	238.23		1.483		-0.12	0.423 Headwall height/system capacity
111	237.9		2.584			
386	238.41		1.023		0.07	0 None
387	238.17		0		0.29	0 None
385	237.76		0			
376	237.85		0.37		0.22	0 None
377	237.5		1.35			
215	238.28		0.214		-0.05	0.123 Headwall height/system capacity
217a	237.57		0		0.56	None
214a	237.5		0		0.6	None
379	237.27		0.475			
160	237.94		0.1		0.31	0 None
159	237.7		1.077			
98a	238.41		0.734		-0.11	0.341 Headwall height/system capacity
99	237.95		1.802			
446	238.17		0.2		0.87	0 None
446a	238.04		0.241			
303	237.85		0.153		0.05	0 None
308a	237.65		0			
48	238.24		0.187		0.31	0 None
49	237.76		0			
313	238.23		0.185		0.19	0 None
314	238.12		0			
414	238.73		0.241		0.16	0 None
415	238.25		0			
276	239.15		0.384			
275	239.1		0		0.48	None
274	238.42		0			
325	238.58		0.387			
326	238.52		0		0.36	0 None
327	238.43		0			









## Attachment No. 3

443	603.3	602.63	0	0.1
120	2614.39	2613.39	0	0
121	32622.89	32438.79	0	0.6
126	1546.31	1545.44	0	0.1
129	33984.22	33929.96	0	0.2
147	956.04	955.09	0	0.1
142	15461.1	15416.29	0	0.3
93	867.52	867.19	0	0
94	867.19	867.19	0	0
64	2882.41	2881.54	0	0
65	2881.54	2881.51	0	0
188	5526.66	5521.5	0	0.1
183	5521.5	5521.2	0	0
108	468.95	468.7	0	0.1
109	14402.93	14377.98	0	0.2
198	192.76	192.63	0	0.1
197	14522.49	14506.05	0	0.1
178	10449.57	10432.78	0	0.2
177	10432.78	10432.17	0	0
219	1393.97	1393.42	0	0
901	1393.42	1393.42	0	0
40	3104.36	3103.62	0	0
41	3103.62	3103.62	0	0
75	754.58	754.25	0	0
74	754.25	754.25	0	0
307	700.43	700.23	0	0
304	700.23	700.23	0	0
341	481.97	481.17	0	0.2
342	481.17	480	0	0.2
343	480	480	0	0
336	8136.52	8102.57	0	0.4
337	8455.5	8278.71	0	2.1
338	5506.48	5489.74	0	0.3
345	5969.75	5967.3	0	0
4	1773.04	1772.19	0	0
393	1772.19	1771.7	0	0
392	1771.7	1771.7	0	0
3	0	0	0	0
394	0	0	0	0
110	14377.98	14371.82	0	0
111	30023.73	30009.47	0	0
386	6112.16	6108.3	0	0.1
387	6108.3	6106.7	0	0
385	6106.7	6106.65	0	0
376	2181.09	2180.47	0	0
377	14340.68	14329.88	0	0.1
215	1171.64	1170.04	0	0.1
217a	8754.39	8750.89	0	0
214a	8750.89	8731.23	0	0.2
379	12160.93	12160.24	0	0
160	695.73	695.41	0	0
159	11127.56	11082.61	0	0.4
98a	5514.98	5509.37	0	0.1
99	15707.95	15651.85	0	0.4
446	1313.51	1312.94	0	0
446a	2871.21	2871.18	0	0
303	906.19	905.85	0	0
308a	905.85	905.85	0	0
48	1521.3	1520.57	0	0
49	1520.57	1520.56	0	0
313	1124.57	1124.36	0	0
314	1124.36	1124.36	0	0
414	1559.37	1558.28	0	0.1
415	1558.28	1558.28	0	0
276	3649.17	3640.72	0	0.2
275	5973.82	5955.64	0	0.3
274	5955.64	5955.45	0	0
325	3153.57	3153.02	0	0
326	3153.02	3151.52	0	0
327	3151.52	3151.5	0	0
OUTLET 05	3357.88	3357.88	0	0
2\4	1751.54	1751.54	0	0
71	1751.54	1751.54	0	0
Node15a	5340.03	5339.77	0	0
Outlet 67	5339.77	5339.77	0	0
Node 22\3	41384.27	40549	0	2
Node 314a	3995.55	3994.77	0	0
OUTLET 34	20454.44	20454.44	0	0
OUTLET 38	6106.65	6106.65	0	0
OUTLET 41	3103.62	3103.62	0	0
OUTLET 49	1520.56	1520.56	0	0
61\2	40360.93	40331.53	0	0.1
Node 74a	1693	1692.99	0	0
OUTLET 74	1692.99	1692.99	0	0
Node 93a	434.72	434.72	0	0
OUTLET 12	49247.73	49247.73	0	0
9	13158.28	13415.44	0	-2
8	3372.83	3368.61	0	0.1
6	3368.61	3358.41	0	0.3
5	3358.41	3357.88	0	0
Pit12	3138.48	3137.36	0	0
Pit13	6298.97	6285.05	0	0.2
Pit32	5073.12	5064.4	0	0.2
SchoolOval	9876.21	9876.15	0.08	0
Pit29	9876.15	9865.53	0	0.1

## Attachment No. 3

Pit34	11733.9	11718.17	0	0.1
Pit GPT	10779.42	10782.49	0	0
discharge f	10782.49	10782.49	0	0
O Pit GPT	0	0	0	0
O 392	1771.7	1771.7	0	0
PaytenParl	5219.69	0	4487.86	14
DundasPar	38783.27	5341.64	31473.49	5.1
401	5341.63	5219.69	0	2.3
Pit6	3165.68	3164.67	0	0
Pit8	3164.68	3161.62	0	0.1
U/S Railway	51245.67	34301.79	16581.35	0.7
D/S Railway	35120.63	32614.08	2241.22	0.8
N172535	11210.77	11210.77	0	0
N101397	14187.34	14187.34	0	0
N101398	20893.67	20893.67	0	0
273	2451.86	2449.1	0	0.1
347	14055.72	14047.6	0	0.1
346	14651.23	14642.11	0	0.1
348	20609.38	20454.44	0	0.8
291	2119.88	2118.64	0	0.1
290	2118.64	2118.53	0	0
278	3259.1	3257.19	0	0.1
279	3257.19	3257.03	0	0
192	4936.3	4933.13	0	0.1
183a	10454.34	10449.57	0	0
200	2028.9	2027.51	0	0.1
199	2027.51	2027.36	0	0
203	5151.04	5145.99	0	0.1
202	5145.99	5145.63	0	0
207	8097.97	8089.85	0	0.1
206	8089.85	8085.36	0	0.1
218a	8085.36	8071.23	0	0.2
209a	8071.23	8055.13	0	0.2
213	8055.13	8035.64	0	0.2
216a	8035.64	8014.37	0	0.3
N160610	40549	40360.93	0	0.5
N168004	10198.6	10198.6	0	0
140	15372.65	15319.12	0	0.3
141	15319.12	15317.77	0	0
131	15416.29	15373.83	0	0.3
130	15373.82	15372.65	0	0
HW47	14187.34	14129.44	0	0.4
N184609	35023.14	34888.29	0	0.4

Run Log for 055 run at 09:24:19 on 1/6/2021 using version 2020.061

The maximum water level in these storages exceeds the maximum elevation you specified: U/S Railway.

DRAINS has extrapolated the Elevation vs Storage table to a higher Elevation. Please provide accurate values for higher elevations.

Upwelling occurred at: 242, 243, Pit8, Pit13, 248, 9

Freeboard was less than 0.15m at 393, 346

The maximum flow in these overflow routes is unsafe: OF149261, OF146119, OF146104, OF146085, OF146067, OF121903, OF121886, OF118839, F 279, F 290, OF82144, OF82141, OF137218, OF78192, OF7815- These sag pits have unsafe water levels for minor storms: 346

These overflow routes carried water uphill (adding energy): OF51139, F 371, F 395, F 9, F Pit8. These results may be invalid. This is likely due to either incorrect surface levels specified at pits or high downstream elevations.

IGNORE THESE WARNINGS AT YOUR OWN PERIL\cf1

## **Attachment No. 3**

4, OF51143, OF51139, OF51134, OF51128, 61\1, F 346, F 337, F 371, F 249, F 253, F 336, F 110, F 98a, F 5, F 142, F 157, F 109, F 177, F 159, F 183, F 197, F 2\4, F 71, F Node15a, F 274, F 239, F 257, F 22\2, F Node  
am tailwater levels which the Lite Hydraulic model cannot handle. Analysing the latter requires solving the full unsteady flow equations in overflow routes using the Full Unsteady Hydraulic model (Formerly Pre

## **Attachment No. 3**

:22\3, F 314, F Node 314a, F 327, F 345, F 348, F 374, F 379, F 377, F 385, F 392, F 395, F 401, F 41, F 415, F 446a, F 445, F 49, F 61\2, F 65, F Node 74a, F 901, F 99, F 111, F 121, F 129, F 9, F Pit6, F Pit12, F Pit34, I  
eimum Hydraulic model).

## **Attachment No. 3**

F Pit13, F Pit8

*B2. DRAINS model results for the proposed Option 1 under 5yr ARI design event*

# Attachment No. 3

DRAINS results prepared from Version 2020.061

PIT / NODE DETAILS	Version 8						
	Name	Max HGL	Max Pond	Max Surfac	Max Pond	Min	Overflow
	HGL			Flow Arriv	Volume	Freeboard	(cu.m/s)
				(cu.m/s)	(cu.m)	(m)	
66	238.17		0.098		0	0.001	Headwall height/system capacity
67	237.63		0.001				
22\1	239.24		0.078		0.2	0	None
22\2	238.7		0				
373	237.63		0.144		0.18	0	None
374	237.38		0				
249	238.21		0.494		0.06	0	None
248	238.12		0		0.1	0	None
244	238.08		0.484				
243	238.04	238.03	0	0.1	0	0	Outlet System
242	237.92		0		0.21	0	None
239	237.78		0				
253	237.86		0.914		0.15	0	None
257	237.7		1.236				
158	238.05		1.174		0.25	0	None
157	237.81		0				
433	238.6		0.078		0.3	0	None
445a	238.32		0		0.77	0	None
445	238.23		0				
443	238.77		0.124		0.12	0	None
120	237.67		0.278		0.27	0	None
121	237.53		3.639				
126	237.67		0.169		0.16	0	None
129	237.58		3.913				
147	237.94		0.175		0.21	0	None
142	237.67		1.731				
93	241.21		0.131		0.32	0	None
94	240.59		0				
64	237.8		0.348		0.27	0	None
65	237.61		0				
188	238.36		0.569		0.04	0	None
183	238.13		0				
108	237.72		0.086		0.17	0	None
109	237.61		1.435				
198	238.18		0.049		0.28	0	None
197	238.09		1.711				
178	238.35		1.077		0.02	0	None
177	237.85		0				
219	237.72		0.229		0.12	0	None
901	237.28		0				
40	237.85		0.469		0.03	0	None
41	237.66		0				
75	238.02		0.138		0.12	0	None
74	237.75		0				
307	237.78		0.144		0.16	0	None
304	237.7		0				
341	238.78		0.122		0.32	0	None
342	238.59		0		0.9		None
343	238.13		0				
336	238.22		2.935		0.24	0	None
337	237.47		0.051		1.92	0.01	Inlet Capacity
338	237.23		0		2.22	0	None
345	236.4		0.122				
4	238.71		0.278		0.26	0	None
393	238.33	238.45	0	0	0.13	0	None
392	238		0				
3	238.62		0		0.75	0	None
394	238.41		0				
110	238.04		1.483		0.07	0	None
111	237.82		2.174				
386	238.21		1.023		0.27	0	None
387	237.91		0		0.56	0	None
385	237.67		0				
376	237.5		0.144		0.21	0	None
377	237.38		1.573				
215	238.13		0.214		0.1	0	None
217a	237.61		0		0.52		None
214a	237.53		0		0.57		None
379	237.29		0.357				
160	237.94		0.1		0.31	0	None
159	237.7		1.077				
98a	238.26		0.734		0.05	0	None
99	237.87		1.471				
446	238.17		0.2		0.87	0	None
446a	238.04		0.241				
303	237.85		0.153		0.05	0	None
308a	237.65		0				
48	238.24		0.187		0.31	0	None
49	237.76		0				
313	238.23		0.185		0.19	0	None
314	238.12		0				
414	238.73		0.241		0.16	0	None
415	238.25		0				
276	239.33		0.384				
275	239.28		0		0.3		None
274	238.44		0				
325	238.58		0.387				
326	238.52		0		0.36	0	None
327	238.43		0				



# Attachment No. 3

Ext2a	3.384	2.398	1.007	64.25	502.05	0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 273	0.384	0.33	0.056	92.8	182.33	0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 347	0.199	0.186	0.013	90.58	177.98	0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 346	0.099	0.093	0.007	90.58	177.98	0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C291	0.218	0.205	0.014	163.99	322.06	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 278	0.335	0.315	0.022	163.99	322.06	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 192	0.173	0.162	0.011	163.99	322.06	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 200	0.199	0.187	0.013	173.54	340.8	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 203	0.306	0.287	0.02	173.54	340.8	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 207	0.289	0.272	0.019	173.54	340.8	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 395	0.089	0.084	0.006	154.05	302.55	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
Ext2b	0.482	0.336	0.146	28.12	219.73	0 AR&R 5 year, 30 minutes storm, average 52.4 mm/h, Zone 2
Ext3	1.471	1.042	0.438	87.7	685.29	0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2

Outflow Volumes for Total Catchment (338 impervious + 745 pervious = 1083 total ha)

Storm	Total Rainf	Total Runo	Impervious	Pervious	Runoff
	cu.m	cu.m	(Runoff cu.m)	(Runoff cu.m)	(Runoff %)
AR&R 5 ye:	105636.5	29505.33	( 29326.82	( 178.51	( 0.2%)
AR&R 5 ye:	159753.7	46859.27	( 45749.73	( 1109.55	( 1.0%)
AR&R 5 ye:	200925.8	60429.38	( 57923.57	( 2505.81	( 1.8%)
AR&R 5 ye:	233735.4	71149.49	( 67057.59	( 14091.91	( 2.5%)
AR&R 5 ye:	283666.8	86929.34	( 79442.00	( 14787.33	( 3.8%)
AR&R 5 ye:	336007.1	108547.88	( 96071.26	( 12476.62	( 5.5%)
AR&R 5 ye:	373579.6	125539.02	( 108878.00	( 16660.95	( 6.5%)
AR&R 5 ye:	426217.5	150598.97	( 126961.52	( 23637.45	( 8.1%)
AR&R 5 ye:	463312	166546.89	( 139489.83	( 27057.07	( 8.5%)
AR&R 5 ye:	516040	186695.05	( 157035.97	( 29659.07	( 8.4%)
AR&R 5 ye:	571492.3	204604.53	( 174760.63	( 29843.90	( 7.6%)
AR&R 5 ye:	614116.8	216678.44	( 188210.88	( 28467.56	( 6.7%)

#### PIPE DETAILS

Name	Max Q (cu.m/s)	Max V (m/s)	Max U/S HGL (m)	Max D/S HGL (m)	Due to Storm
P 66	0.096	1.58	237.954	237.95	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 22\1	0.078	1.75	239.069	238.859	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 373	0.144	1.17	237.523	237.379	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 249	0.553	0.52	238.117	238.117	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 248	0.461	0.36	238.106	238.083	AR&R 5 year, 45 minutes storm, average 41.4 mm/h, Zone 2
P 244	0.916	0.85	238.083	238.041	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 243	0.914	0.99	238.01	237.916	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P 242	0.914	1.55	237.824	237.777	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P 253	0.913	1.35	237.747	237.697	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P 158	1.174	1.81	237.859	237.808	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 433	0.078	0.59	238.595	238.321	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 445a	0.2	1.35	238.26	238.234	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 443	0.125	1.1	238.665	238.321	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 120	0.278	1.31	237.567	237.527	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 126	0.169	1.31	237.541	237.579	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 147	0.175	1.32	237.83	237.666	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 93	0.13	1.94	240.59	240.592	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 64	0.348	1.41	237.678	237.612	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 188	0.569	1.55	238.192	238.126	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 108	0.086	1.04	237.65	237.609	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 198	0.049	0.93	238.129	238.09	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 178	1.077	1.62	238.063	238.354	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 219	0.229	1.68	237.579	237.283	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 40	0.469	1.56	237.68	237.658	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 75	0.138	1.44	237.899	237.749	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 307	0.144	1.14	237.697	237.705	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 341	0.122	1.22	238.651	238.586	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 342	0.122	1.36	238.562	238.133	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 336	2.873	1.61	238.034	237.467	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 337	2.896	2.45	237.205	237.226	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 338	2.896	2.57	237.126	236.397	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 4	0.277	1.74	238.496	238.326	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 393	0.277	2.07	238.2	238.005	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 3	0	0	238.625	238.407	AR&R 5 year, 5 minutes storm, average 117 mm/h, Zone 2
P 110	1.483	1.69	237.852	237.822	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 386	1.021	2.11	237.877	237.906	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 387	1.021	1.82	237.803	237.674	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 376	0.144	1.24	237.386	237.38	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 215	0.214	1.2	237.868	237.918	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 217a	0.992	1.14	237.536	237.527	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 214a	0.992	1.65	237.43	237.293	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 160	0.1	1.14	237.874	237.699	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 98a	0.733	1.85	238.065	237.865	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 446	0.2	1.13	238.051	238.041	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 303	0.153	1.36	237.722	237.65	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 48	0.187	2.14	238.002	237.763	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 313	0.185	1.26	238.137	238.124	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 414	0.241	1.61	238.593	238.249	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 276	0.384	0.64	239.33	239.281	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 275	0.767	1.89	239.147	238.436	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 325	0.387	1.57	238.585	238.518	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 326	0.387	1.47	238.44	238.433	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 9	0.182	1.23	238.521	238.441	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 8	0.197	1.76	238.381	238.051	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 6	0.184	1.35	237.982	237.923	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P Pit12	0.404	0.75	237.74	237.738	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P Pit13	1.409	1.3	237.612	237.318	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P Pit32	1.408	1.3	237.193	237.049	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P Basin-pit	1.415	1.31	236.927	236.797	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P Pit29	1.415	1.31	236.662	236.356	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P Pit34	1.434	2.22	236.159	236.051	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2

## Attachment No. 3

P Pit GPT	1.458	2.74	235.892	234.672	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P 400	1.374	1.43	238.15	238.04	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P Pit6	0.012	0.02	237.885	237.88	AR&R 5 year, 4.5 hours storm, average 11.7 mm/h, Zone 2
P Pit8	1.335	1.24	237.787	237.738	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P 371	2.26	1.73	237.934	237.854	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 273	0.384	1.21	239.331	239.281	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 347	0.209	1.13	237.768	237.526	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 346	0.319	2.15	237.45	236.636	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 291	0.218	1.53	238.551	238.508	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 278	0.335	1.49	238.677	238.567	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 192	0.459	1.55	238.146	238.066	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 200	0.199	1.48	238.384	238.384	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 203	0.505	1.57	237.981	237.866	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 207	0.795	0.57	237.827	237.818	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 208	0.795	0.57	237.793	237.792	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 218a	0.806	0.58	237.773	237.756	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 209a	0.806	0.58	237.74	237.73	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 213	0.798	0.83	237.687	237.648	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 216a	0.8	0.79	237.61	237.608	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
Pipe11344:	2.26	1.55	237.744	237.644	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 140	0.456	1.55	237.325	237.245	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 131	0.79	1.96	237.482	237.452	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
Pipe10930:	3.381	1.26	238.562	238.512	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2

### CHANNEL DETAILS

Name	Max Q (cu.m/s)	Max V (m/s)	Due to Storm
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### OVERFLOW ROUTE DETAILS

Name	Max Q U/S	Max Q D/S	Safe Q	Max D	Max DxV	Max Width	Max V	Due to Storm
F 66	0.001	0.001	0	0.029	0.01	0.33	0.26	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 67	0.098	0.098	0	0.124	0.07	3.26	0.59	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 22\1	0	0	0	0	0	0	0	
F 22\2	0.078	0.078	0	0.115	0.07	2.96	0.57	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 373	0	0	0	0	0	0	0	
F 374	0.144	0.144	0	0.141	0.09	3.83	0.64	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 249	0	0	0	0	0	0	0	
OF51143	0	0	0	0	0	0	0	
OF51134	0	0	0	0	0	0	0	
OF51139	0	0	0	0	0	0	0	
F 239	0.914	0.914	0	0.257	0.27	7.02	1.05	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
F 253	0	0	0	0	0	0	0	
F 257	2.122	2.122	0	0.343	0.47	8.85	1.36	AR&R 5 year, 45 minutes storm, average 41.4 mm/h, Zone 2
F 158	0	0	0	0	0	0	0	
F 157	1.174	1.174	0	0.279	0.32	7.58	1.14	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 443	0	0	0	0	0	0	0	
F 445a	0	0	0	0	0	0	0	
F 445	0.2	0.2	0	0.159	0.1	4.58	0.66	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 443	0	0	0	0	0	0	0	
F 120	0	0	0	0	0	0	0	
F 121	3.913	3.913	0	0.426	0.72	8.85	1.7	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 126	0	0	0	0	0	0	0	
F 129	4.079	4.079	0	0.433	0.75	8.85	1.73	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 147	0	0	0	0	0	0	0	
F 142	1.901	1.901	0	0.331	0.43	8.85	1.3	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 93	0	0	0	0	0	0	0	
F 94	0.13	0.13	0	0.136	0.09	3.68	0.62	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 64	0	0	0	0	0	0	0	
F 65	0.348	0.348	0	0.189	0.15	5.32	0.77	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 188	0	0	0	0	0	0	0	
OF118839	0.569	0.569	0	0.22	0.2	6.1	0.91	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 108	0	0	0	0	0	0	0	
F 109	1.483	1.483	0	0.303	0.37	8.17	1.22	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 198	0	0	0	0	0	0	0	
F 197	1.731	1.731	0	0.32	0.41	8.59	1.27	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 178	0	0	7.7E+14	0	0	0	0	
F 177	1.077	1.077	0	0.271	0.3	7.39	1.1	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF51117	0	0	0	0	0	0	0	
F 901	0.229	0.229	0	0.166	0.11	4.76	0.68	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 40	0	0	0	0	0	0	0	
F 41	0.469	0.469	0	0.207	0.18	5.77	0.85	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 75	0	0	0	0	0	0	0	
F 74	0.138	0.138	0	0.138	0.09	3.75	0.64	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 307	0	0	0	0	0	0	0	
F 341	0	0	0	0	0	0	0	
F 343	0.122	0.122	0	0.133	0.08	3.58	0.62	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 336	0	0	0	0	0	0	0	
F 337	0.01	0.01	0	0.06	0.02	1.12	0.41	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 338	0	0	0	0	0	0	0	
F 345	2.903	2.903	0	0.382	0.58	8.85	1.52	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 4	0	0	0	0	0	0	0	
F 393	0	0	0	0	0	0	0	
F 392	0.277	0.277	0	0.176	0.13	5	0.72	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 3	0	0	0	0	0	0	0	
F 394	0	0	0	0	0	0	0	
F 110	0	0	0	0	0	0	0	
F 111	3.639	3.639	0	0.415	0.69	8.85	1.65	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 386	0	0	0	0	0	0	0	
OF51147	0	0	0	0	0	0	0	
F 385	1.021	1.021	0	0.266	0.29	7.26	1.09	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 376	0	0	0	0	0	0	0	
F 377	1.711	1.711	0	0.319	0.4	8.57	1.27	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 215	0	0	0	0	0	0	0	
F 379	1.345	1.345	0	0.293	0.35	7.92	1.18	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 160	0	0	0	0	0	0	0	

### Attachment No. 3

F 159	1.174	1.174	0	0.279	0.32	7.58	1.14 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 98a	0	0	0	0	0	0	0
F 99	2.174	2.174	0	0.346	0.47	8.85	1.37 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 446	0	0	0	0	0	0	0
F 446a	0.435	0.435	0	0.202	0.17	5.65	0.83 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 303	0	0	0	0	0	0	0
F 48	0	0	0	0	0	0	0
F 49	0.187	0.187	0	0.156	0.1	4.5	0.65 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 313	0	0	0	0	0	0	0
F 314	0.185	0.185	0	0.156	0.1	4.5	0.64 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 414	0	0	0	0	0	0	0
F 415	0.241	0.241	0	0.169	0.12	4.82	0.69 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 274	0.767	0.767	0	0.242	0.24	6.66	1 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
OF51150	0	0	0	0	0	0	0
F 327	0.387	0.387	0	0.195	0.16	5.46	0.8 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 2\4	0.281	0.281	0	0.177	0.13	5.01	0.73 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 71	0.281	0.281	0	0.177	0.13	5.01	0.73 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Node15a	0.724	0.724	0	0.238	0.23	6.54	0.98 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Node 2\	1.236	1.236	0	0.284	0.33	7.71	1.15 AR&R 5 year, 45 minutes storm, average 41.4 mm/h, Zone 2
F Node 31\	0.615	0.615	0	0.225	0.21	6.24	0.93 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 61\2	0	0	0	0	0	0	0
F Node 74\	0.3	0.3	0	0.181	0.13	5.11	0.74 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Node 93\	0.061	0.061	0	0.106	0.06	2.68	0.54 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 9	0	0	0	0	0	0	0
F 8	0	0	0	0	0	0	0
F 6	0	0	0	0	0	0	0
F 5	0.184	0.184	0	0.155	0.1	4.48	0.65 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Pit12	0	0	0	0	0	0	0
F Pit13	0.002	0.002	0	0.008	0	5.04	0.11 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
F Pit32	0	0	0	0	0	0	0
OF78192	0	0	0	0	0	0	0
F Pit29	0	0	0	0	0	0	0
F Pit34	0.166	0.166	0	0.147	0.1	4.05	0.66 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Pit GPT	0	0	0	0	0	0	0
OF78154	1.352	1.352	0	0.136	0.1	20.14	0.77 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
OF51128	0	0	0	0	0	0	0
F 401	1.374	1.374	0	0.295	0.35	7.98	1.19 AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
F Pit6	0	0	0	0	0	0	0
F Pit8	0.152	0.152	0	0.143	0.09	3.92	0.64 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
F 371	0	0	0	0	0	0	0
OF153962	2.26	2.26	0	0.5	1.13	4	2.26 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
OF137218	2.269	2.571	0	0.18	0.17	22.81	0.95 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF82141	4.13	4.13	0	0.423	0.41	10	0.98 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF82144	3.384	3.384	0	0.374	0.34	10	0.9 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 273	0	0	0	0	0	0	0
F 347	0	0	0	0	0	0	0
F 346	0	0	0	0	0	0	0
F 348	2.972	2.972	-4E+28	0.385	0.59	8.85	1.54 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 291	0	0	0	0	0	0	0
F 290	0.218	0.218	0	0.164	0.11	4.69	0.67 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF118859	0	0	0	0	0	0	0
F 279	0.335	0.335	0	0.187	0.14	5.27	0.76 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF118882	0.049	0.049	0	0.099	0.05	2.44	0.52 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 183	1.077	1.077	0	0.271	0.3	7.39	1.1 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF121852	0	0	0	0	0	0	0
OF121886	0.199	0.199	0	0.159	0.1	4.58	0.66 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF121872	0	0	0	0	0	0	0
OF121903	0.505	0.505	0	0.211	0.19	5.88	0.88 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF121924	0	0	0	0	0	0	0
OF153967	0	0	0	0	0	0	0
F 395	2.348	2.348	0	0.355	0.5	8.85	1.41 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
OF51123	0	0	0	0	0	0	0
OF218457	0.482	0.482	0	0.209	0.18	5.81	0.86 AR&R 5 year, 30 minutes storm, average 52.4 mm/h, Zone 2
OF135437	1.471	1.471	0	0.224	0.15	10	0.66 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146104	1.445	1.445	0	0.3	0.36	8.1	1.21 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146119	1.901	1.901	0	0.331	0.43	8.85	1.3 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146085	1.111	1.111	0	0.274	0.31	7.46	1.12 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146067	1.901	1.901	0	0.331	0.43	8.85	1.3 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF149269	0	0	0	0	0	0	0
OF205948	3.381	3.381	0	0.374	0.34	10	0.9 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2

#### DETENTION BASIN DETAILS

Name	Max WL	MaxVol	Max Q	Max Q Total	Max Q Low Level	Max Q High Level
SchoolOval	237.05	575.6	1.415	1.415	0	
PaytenParl	238.23	11896.3	1.352	0	1.352	
DundasPar	238.36	18676.2	1.374	1.374	0	
U/S Railwa	238.09	2925.1	2.26	2.26	0	

CONTINUITY CHECK for AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2

Node	Inflow	Outflow	Storage	Ch Diference
(cu.m)	(cu.m)	(cu.m)	(cu.m)	%
66	707.11	707.32	0	0
67	707.32	707.32	0	0
22\1	371.87	371.47	0	0.1
22\2	371.47	371.47	0	0
373	788.1	787.52	0	0.1
374	787.52	787.52	0	0
249	4027.76	3766.45	0	6.5
248	3766.45	3747.99	0	0.5
244	7985.27	7943.47	0	0.5
243	7943.47	7933.4	0	0.1
242	7933.4	7922.29	0	0.1
239	7922.29	7922.07	0	0

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253	7922.07	7893.14	0	0.4
257	16476.87	16460.46	0	0.1
158	11080.63	11063.91	0	0.2
157	11063.91	11063.06	0	0
433	713.47	712.29	0	0.2
445a	1314.64	1311.91	0	0.2
445	1311.91	1311.88	0	0
443	603.3	602.35	0	0.2
120	2614.39	2613.22	0	0
121	32615.12	32430.75	0	0.6
126	1546.31	1545.33	0	0.1
128	33976.07	33921.71	0	0.2
147	956.04	954.75	0	0.1
142	15434.84	15390.04	0	0.3
93	867.52	867.06	0	0.1
94	867.06	867.06	0	0
64	2882.41	2881.32	0	0
65	2881.32	2881.29	0	0
188	5526.45	5520.57	0	0.1
183	5520.57	5520.28	0	0
108	468.95	468.61	0	0.1
109	14400.03	14375.06	0	0.2
198	192.76	192.58	0	0.1
197	14496.57	14480.14	0	0.1
178	10448	10430.89	0	0.2
177	10430.89	10430.29	0	0
219	1393.97	1393.17	0	0.1
901	1393.17	1393.17	0	0
40	3104.36	3103.38	0	0
41	3103.38	3103.38	0	0
75	754.58	754.14	0	0.1
74	754.14	754.14	0	0
307	700.43	700.18	0	0
304	700.18	700.18	0	0
341	481.97	480.83	0	0.2
342	480.83	479.11	0	0.4
343	479.11	479.11	0	0
336	19347.24	19232.5	0	0.6
337	19585.4	19537.34	0	0.2
338	19476.24	19446.18	0	0.2
345	19925.29	19921.67	0	0
4	1773.04	1772.58	0	0
393	1772.58	1771.24	0	0.1
392	1771.24	1771.24	0	0
3	0	0	0	0
394	0	0	0	0
110	14375.06	14366.33	0	0.1
111	30016.15	30001.89	0	0
386	6109.97	6102.61	0	0.1
387	6102.61	6102.93	0	0
385	6102.93	6102.89	0	0
376	787.52	787.12	0	0.1
377	14314.75	14304.01	0	0.1
215	1171.64	1167.99	0	0.3
217a	9165.21	9160.75	0	0
214a	9160.75	9135.58	0	0.3
379	12135.27	12134.57	0	0
160	695.73	695.3	0	0.1
155	11125.57	11080.63	0	0.4
98a	5514.4	5507.36	0	0.1
99	15705.95	15649.84	0	0.4
446	1311.88	1311.21	0	0.1
446a	2869.09	2869.07	0	0
303	906.19	905.73	0	0.1
308a	905.73	905.73	0	0
48	1521.3	1520.41	0	0.1
49	1520.41	1520.4	0	0
313	1124.57	1124.3	0	0
314	1124.3	1124.3	0	0
414	1559.37	1557.89	0	0.1
415	1557.89	1557.89	0	0
276	3649.17	3640.66	0	0.2
275	6089.83	6067.23	0	0.4
274	6067.23	6067.04	0	0
325	3153.57	3152.87	0	0
326	3152.87	3151.06	0	0.1
327	3151.06	3151.04	0	0
OUTLET 05	1965.96	1965.96	0	0
2\4	1751.54	1751.54	0	0
71	1751.54	1751.54	0	0
Node15a	5340.13	5339.89	0	0
Outlet 67	5339.89	5339.89	0	0
Node 22\3	8615.08	8583.72	0	0.4
Node 314a	3993.36	3992.58	0	0
OUTLET 34	21546	21546	0	0
OUTLET 38	6102.89	6102.89	0	0
OUTLET 41	3103.38	3103.38	0	0
OUTLET 49	1520.4	1520.4	0	0
61\2	0	0	0	0
Node 74a	1692.88	1692.88	0	0
OUTLET 74	1692.88	1692.88	0	0
Node 93a	434.72	434.72	0	0
OUTLET 12	49210.01	49210.01	0	0
9	1947.49	1942.94	0	0.2
8	1976.51	1972.38	0	0.2

## Attachment No. 3

6	1972.38	1966.16	0	0.3
5	1966.16	1965.96	0	0
Pit12	3138.48	3135.32	0	0.1
Pit13	3910.49	3840.82	0.16	1.8
Pit32	3840.82	3766.89	0	1.9
SchoolOval	7366.77	7326.69	40.11	0
Pit29	7326.69	7256.77	0	1
Pit34	9125.14	9051.38	0	0.8
Pit GPT	8112.63	8061.68	0	0.6
discharge f	8061.68	8061.68	0	0
O Pit GPT	0	0	0	0
O 392	1771.24	1771.24	0	0
PaytenParl	12050.2	806.6	10861.02	3.2
DundasPar	25915.43	8892.09	16826.29	0.8
401	8892.09	8884.52	0	0.1
Pit6	0	-4.23	0	0
Pit8	802.37	775.18	0	3.4
U/S Railwa	19723.13	18967.5	719.72	0.2
N192692	18967.5	18965.77	0	0
N172535	11210.77	11210.77	0	0
N101397	33289.33	32794.09	0	1.5
N101398	19253.74	19253.74	0	0
273	2451.86	2449.17	0	0.1
347	1268.42	1267.82	0	0
346	1871.48	1871.84	0	0
348	21793.45	21546	0	1.1
291	2119.88	2118.44	0	0.1
290	2118.44	2118.33	0	0
278	3259.1	3256.88	0	0.1
279	3256.88	3256.71	0	0
192	4935.99	4932.51	0	0.1
183a	10452.79	10448	0	0
200	2028.9	2027.32	0	0.1
199	2027.32	2027.18	0	0
203	5150.86	5145.35	0	0.1
202	5145.35	5144.98	0	0
207	8097.33	8087.6	0	0.1
208	8087.6	8083.72	0	0
218a	8083.72	8067.57	0	0.2
209a	8067.57	8049.43	0	0.2
213	8049.43	8024.55	0	0.3
216a	8024.55	7997.22	0	0.3
HW1103	18965.77	18929.11	0	0.2
N192689	19748.05	19746.19	0	0
N162485	32794.09	32794.09	0	0
61\1	0	0	0	0
N270468	2176.58	2176.58	0	0
N168004	10198.6	10198.6	0	0
*(11x) 1.5x	0	0	0	0
140	15345.62	15289.7	0	0.4
141	15289.69	15288.35	0	0
131	15390.04	15346.82	0	0.3
130	15346.82	15345.62	0	0
HW47	19253.74	19103.06	0	0.8
N184609	19103.06	19102	0	0

Run Log for 055 run at 09:29:38 on 1/6/2021 using version 2020.061

No water upwelling from any pit.

Freeboard was less than 0.15m at 243, Pit8, Pit13, 248, 393

The maximum flow in these overflow routes is unsafe: OF205948, OF146119, OF146104, OF146085, OF146067, OF218457, OF153962, OF121903, OF121886, OF118839, F 279, F 290, OF82144, OF82141, OF137.

These overflow routes carried water uphill (adding energy): F Pit8. These results may be invalid. This is likely due to either incorrect surface levels specified at pits or high downstream tailwater levels which th

IGNORE THESE WARNINGS AT YOUR OWN PERIL.\cf1

## **Attachment No. 3**

218, OF78154, F 5, F 142, F 157, F 109, F 177, F 159, F 183, F 197, F 2\4, F 71, F Node15a, F 274, F 239, F 257, F Node 22\3, F 314, F Node 314a, F 327, F 345, F 348, F 374, F 379, F 377, F 385, F 392, F 395, F 401, F  
e Lite Hydraulic model cannot handle. Analysing the latter requires solving the full unsteady flow equations in overflow routes using the Full Unsteady Hydraulic model (Formerly Premium Hydraulic model).

### **Attachment No. 3**

: 41, F 415, F 446a, F 445, F 49, F 65, F Node 74a, F 901, F 99, F 111, F 121, F 129, F Pit34, F Pit13, F Pit8

*B3. DRAINS model results for the proposed Option 2 under 5yr ARI design event*

# Attachment No. 3

DRAINS results prepared from Version 2020.061

PIT / NODE DETAILS	Version 8						
	Name	Max HGL	Max Pond	Max Surfac	Max Pond	Min	Overflow
	HGL	Flow Arriv	Volume	Freeboard	(cu.m/s)	(m)	Constraint
66	238.17		0.098		0	0.001	Headwall height/system capacity
67	237.63		0.001				
22\1	239.24		0.078		0.2	0	None
22\2	238.7		0				
373	237.63		0.144		0.18	0	None
374	237.38		0				
249	238.21		0.494		0.06	0	None
248	238.12		0		0.1	0	None
244	238.08		0.484				
243	238.04	238.03	0	0.1	0	0	Outlet System
242	237.92		0		0.21	0	None
239	237.78		0				
253	237.56		0		0.45	0	None
257	237.51		0.82				
156	238.05		1.174		0.25	0	None
157	237.81		0				
433	238.6		0.078		0.3	0	None
445a	238.32		0		0.77	0	None
445	238.23		0				
443	238.77		0.124		0.12	0	None
120	237.67		0.278		0.27	0	None
121	237.53		3.639				
126	237.67		0.169		0.16	0	None
129	237.58		3.913				
147	237.94		0.175		0.21	0	Headwall height/system capacity
142	237.67		1.731				
93	241.21		0.131		0.32	0	None
94	240.59		0				
64	237.8		0.348		0.27	0	Headwall height/system capacity
65	237.61		0				
188	238.36		0.569		0.04	0	Headwall height/system capacity
183	238.13		0				
108	237.72		0.086		0.17	0	None
109	237.61		1.435				
198	238.18		0.049		0.28	0	Headwall height/system capacity
197	238.09		1.711				
178	238.35		1.077		0.02	0	Headwall height/system capacity
177	237.85		0				
219	237.72		0.229		0.12	0	None
901	237.28		0				
40	237.85		0.469		0.03	0	Headwall height/system capacity
41	237.66		0				
75	238.02		0.138		0.12	0	None
74	237.75		0				
307	237.78		0.144		0.16	0	None
304	237.7		0				
341	238.78		0.122		0.6	0	Headwall height/system capacity
342	238.59		0		0.9		None
343	238.13		0				
4	238.71		0.278			0	None
393	238.33	238.45	0	0	0.13	0	None
392	238		0				
3	238.62		0		0.75	0	None
394	238.41		0				
110	238.04		1.483		0.07	0	None
111	237.82		2.174				
386	238.21		1.023		0.27	0	Headwall height/system capacity
387	237.91		0		0.56	0	None
385	237.67		0				
376	237.5		0.144		0.21	0	None
377	237.38		1.573				
215	238.13		0.214		0.1	0	Headwall height/system capacity
217a	237.61		0		0.52		None
214a	237.53		0		0.57		None
379	237.29		0.357				
160	237.94		0.1		0.31	0	Headwall height/system capacity
159	237.7		1.077				
98a	238.26		0.734		0.05	0	Headwall height/system capacity
99	237.87		1.471				
446	238.17		0.2		0.87	0	Headwall height/system capacity
446a	238.04		0.241				
303	237.85		0.153		0.05	0	None
308a	237.65		0				
48	238.24		0.187		0.31	0	Headwall height/system capacity
49	237.76		0				
313	238.23		0.185		0.19	0	Headwall height/system capacity
314	238.12		0				
414	238.73		0.241		0.16	0	Headwall height/system capacity
415	238.25		0				
276	239.33		0.384				
275	239.28		0		0.3		None
274	238.44		0				
325	238.58		0.387				
326	238.52		0		0.36	0	None
327	238.43		0				
9	238.57		0.182		0.21	0	None
8	238.44		0.01		0.74	0	None
6	238.05		0		1.19	0	None
5	237.42		0				

### Attachment No. 3

Pit12	237.79	0.404	0.96	0	None
Pit13	237.79	237.92	0.371	8.6	0
Pit32	237.49		0		1.04
Pit29	236.95		0		1.52
Pit34	236.38		0.292		2.14
Pit GPT	235.97		0		0.166 Inlet Capacity
discharge f	234.63		0		0 None
401	237.98		0		
Pit6	237.89		0		0 None
Pit8	237.89		0.906		0.01
N192692	237.81		0		0.371 Inlet Capacity
273	239.42		0.384		0.07
347	237.86		0.209		1.14
346	237.53	238.03	0.099	1.4	0.44
348	236.64		0.369		0 Inlet Capacity
291	238.71		0.218		0.06
290	238.51		0		0 None
278	238.81		0.335		0.8
279	238.57		0		0 None
192	238.39		0.508		-0.03
183a	238.07		0.618		0.049 Headwall height/system capacity
200	238.55		0.199		0.08
199	238.38		0		0 None
203	238.13		0.505		0.14
202	237.87		0		0 None
207	237.84		0.794		0.16
208	237.82		0		0.18
218a	237.79		0		0.23
209a	237.76		0		0.22
213	237.73		0		0.65
216a	237.65		0		0.39
HW1103	237.83		0.874		0 None
N192689	237.61		0.089		
337	237.6		0.051		1.79
338	237.28		0		0.01 Inlet Capacity
345	236.28		0.122		2.17
N230386	237.72		0		0 None
140	237.79		1.901		-0.19
141	237.25		1.445		1.445 Headwall height/system capacity
131	238.23		1.901		-0.16
130	237.45		1.111		1.111 Headwall height/system capacity
HW47	238.66		3.384		1.64
N184609	238.51		0		0 None

#### SUB-CATCHMENT DETAILS

Name	Max Flow Q (cu.m/s)	Paved Max Q (cu.m/s)	Grassed Max Q (cu.m/s)	Paved Tc (min)	Grassed Tc (min)	Supp. Tc (min)	Due to Storm
C 66	0.098	0.092	0.006	118.64	233.06		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 221	0.078	0.066	0.012	59.11	116.21		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 373	0.144	0.134	0.01	79.86	156.93		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 249	0.494	0.464	0.032	134.93	265.01		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 244	0.484	0.454	0.032	145.76	286.29		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 433	0.078	0.073	0.005	154.05	302.55		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 443	0.124	0.115	0.009	62.86	123.57		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 120	0.278	0.261	0.018	158.08	310.45		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 126	0.169	0.159	0.011	153.03	300.55		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 147	0.175	0.163	0.012	79.86	156.93		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 93	0.069	0.065	0.005	93.13	182.97		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 64	0.348	0.327	0.023	137.14	269.36		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 188	0.351	0.329	0.023	163.99	322.06		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 108	0.086	0.08	0.006	79.86	156.93		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 109	0.263	0.247	0.017	188.1	369.38		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 198	0.049	0.045	0.004	49.27	96.89		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 219	0.229	0.214	0.016	90.58	177.98		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 40	0.469	0.438	0.032	99.28	195.05		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 75	0.138	0.128	0.009	79.86	156.93		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 307	0.144	0.134	0.011	62.86	123.57		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 341	0.122	0.113	0.009	49.27	96.89		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 4	0.278	0.259	0.019	95.62	187.87		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 387	0.41	0.381	0.03	66.93	131.56		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 215	0.214	0.199	0.015	79.86	156.93		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 379	0.357	0.335	0.023	139.33	273.66		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 160	0.1	0.094	0.007	113.7	223.36		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 98a	0.219	0.205	0.014	111.17	218.4		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 303	0.153	0.143	0.01	87.99	172.89		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 49	0.187	0.175	0.012	134.93	265.01		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 313	0.185	0.173	0.013	90.58	177.98		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 414	0.241	0.225	0.016	96.85	190.28		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 276	0.384	0.36	0.025	160.06	314.36		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 325	0.387	0.363	0.025	134.93	265.01		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 24	0.281	0.262	0.019	93.13	182.97		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C Node 93:	0.061	0.057	0.004	116.19	228.24		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 9	0.182	0.17	0.012	184.53	362.38		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 8	0.01	0.009	0.001	36.05	70.94		0 AR&R 5 year, 45 minutes storm, average 41.4 mm/h, Zone 2
C Pit12	0.404	0.379	0.026	128.12	251.67		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C Basin-pit	0.507	0.476	0.033	116.19	228.24		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C Pit34	0.292	0.273	0.02	95.62	187.87		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C Pit6	0.383	0.359	0.025	137.14	269.36		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 400	0.692	0.649	0.045	148.91	292.46		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 371	0.478	0.448	0.031	111.17	218.4		0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
Ind1	2.269	2.17	0.101	64.25	502.05		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
Ext1	2.493	1.767	0.742	64.25	502.05		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
Ext2a	3.384	2.398	1.007	64.25	502.05		0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
C 273	0.384	0.33	0.056	92.8	182.33		0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2

## Attachment No. 3

C 347	0.199	0.186	0.013	90.58	177.98	0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 346	0.099	0.093	0.007	90.58	177.98	0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C291	0.218	0.205	0.014	163.99	322.06	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 278	0.335	0.315	0.022	163.99	322.06	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 192	0.173	0.162	0.011	163.99	322.06	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 200	0.199	0.187	0.013	173.54	340.8	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 203	0.306	0.287	0.02	173.54	340.8	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 207	0.289	0.272	0.019	173.54	340.8	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
Cat113175	0.089	0.084	0.006	154.05	302.55	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
C 336	0.676	0.58	0.098	229.79	451.21	0 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
C 337	0.051	0.048	0.003	113.7	223.36	0 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
Ext2b	0.482	0.336	0.146	28.12	219.73	0 AR&R 5 year, 30 minutes storm, average 52.4 mm/h, Zone 2
Ext3	1.471	1.042	0.438	87.7	685.29	0 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2

Outflow Volumes for Total Catchment (338 impervious + 745 pervious = 1083 total ha)

Storm	Total Rainfall	Total Runoff	Impervious	Pervious	Runoff
	cu.m	cu.m	(Runoff cu.m)	(Runoff cu.m)	(Runoff %)
AR&R 5 ye:	105636.5	29505.33	( 29326.82 )	( 178.51 )	( 0.2%)
AR&R 5 ye:	159753.7	46859.27	( 45749.73 )	( 1109.55 )	( 1.0%)
AR&R 5 ye:	200925.8	60429.38	( 57923.57 )	( 2505.81 )	( 1.8%)
AR&R 5 ye:	233735.4	71149.49	( 67057.59 )	( 14091.91 )	( 2.5%)
AR&R 5 ye:	283666.8	86929.34	( 79442.00 )	( 7487.33 )	( 3.8%)
AR&R 5 ye:	336007.1	108547.88	( 96071.26 )	( 12476.62 )	( 5.4%)
AR&R 5 ye:	373579.6	125539.02	( 108878.00 )	( 16660.95 )	( 6.5%)
AR&R 5 ye:	426217.5	150598.97	( 126961.52 )	( 23637.45 )	( 8.1%)
AR&R 5 ye:	463312	166546.89	( 139489.83 )	( 27057.07 )	( 8.5%)
AR&R 5 ye:	516040	186695.05	( 157035.97 )	( 29659.07 )	( 8.4%)
AR&R 5 ye:	571492.3	204604.53	( 174760.63 )	( 29843.90 )	( 7.6%)
AR&R 5 ye:	614116.8	216678.44	( 188210.88 )	( 28467.56 )	( 6.7%)

### PIPE DETAILS

Name	Max Q (cu.m/s)	Max V (m/s)	Max U/S HGL (m)	Max D/S HGL (m)	Due to Storm
P 66	0.096	1.58	237.954	237.95	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 22\1	0.078	1.75	239.069	238.859	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 373	0.144	1.17	237.523	237.379	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 249	0.553	0.52	238.117	238.117	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 248	0.461	0.36	238.106	238.083	AR&R 5 year, 45 minutes storm, average 41.4 mm/h, Zone 2
P 244	0.916	0.85	238.083	238.041	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 243	0.914	0.99	238.01	237.916	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P 242	0.914	1.55	237.824	237.777	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P 253	0	0	237.56	237.51	AR&R 5 year, 5 minutes storm, average 117 mm/h, Zone 2
P 158	1.174	1.81	237.859	237.808	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 433	0.078	0.59	238.595	238.321	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 445a	0.2	1.35	238.26	238.234	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 443	0.125	1.1	238.665	238.321	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 120	0.278	1.31	237.567	237.527	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 126	0.169	1.31	237.541	237.579	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 147	0.175	1.32	237.83	237.666	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 93	0.13	1.94	240.59	240.592	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 64	0.348	1.41	237.678	237.612	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 188	0.569	1.55	238.192	238.126	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 108	0.086	1.04	237.65	237.609	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 198	0.049	0.93	238.129	238.09	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 178	1.077	1.62	238.063	238.354	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 219	0.229	1.68	237.579	237.283	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 40	0.469	1.56	237.68	237.658	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 75	0.138	1.44	237.899	237.749	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 307	0.144	1.14	237.697	237.705	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 341	0.122	1.22	238.651	238.586	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 342	0.122	1.36	238.566	238.133	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 4	0.277	1.74	238.496	238.326	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 393	0.277	2.07	238.2	238.005	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 3	0	0	238.625	238.407	AR&R 5 year, 5 minutes storm, average 117 mm/h, Zone 2
P 110	1.483	1.69	237.852	237.822	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 386	1.021	2.11	237.877	237.906	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 387	1.021	1.82	237.803	237.674	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 376	0.144	1.24	237.386	237.38	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 215	0.214	1.2	237.868	237.918	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 217a	0.992	1.14	237.536	237.527	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 214a	1.65	237.43	237.293	237.293	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 160	0.1	1.14	237.874	237.699	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 98a	0.733	1.85	238.065	237.865	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 446	0.2	1.13	238.051	238.041	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 303	0.153	1.36	237.722	237.65	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 48	0.187	2.14	238.002	237.763	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 313	0.185	1.26	238.137	238.124	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 414	0.241	1.61	238.593	238.249	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 276	0.384	0.64	239.33	239.281	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 275	0.767	1.89	239.147	238.436	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 325	0.387	1.57	238.585	238.518	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 326	0.387	1.47	238.44	238.433	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 9	0.182	1.23	238.521	238.441	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 8	0.197	1.76	238.381	238.051	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 6	0.184	1.35	237.982	237.923	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
Pit12	0.403	0.75	237.782	237.786	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
Pit13	0.775	1.43	237.696	237.485	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
Pit32	0.775	1.43	237.395	237.291	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P Basin-pit	0.82	1.52	237.083	236.953	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
Pit29	0.82	1.52	237.677	236.384	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
Pit34	0.854	3.29	236.083	236.035	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
Pit GPT	0.882	2.43	235.844	234.632	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 400	0.953	1.41	238.091	237.981	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
Pit6	0.008	0.02	237.891	237.892	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2

### Attachment No. 3

P Pit8	0.747	1.38	237.823	237.786	AR&R 5 year, 4.5 hours storm, average 11.7 mm/h, Zone 2
P 371	0.874	1.58	237.886	237.806	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P 273	0.384	1.21	239.331	239.281	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 347	0.209	1.13	237.768	237.526	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 346	0.319	2.15	237.45	236.636	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
P 291	0.218	1.53	238.551	238.508	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 278	0.335	1.49	238.677	238.567	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 192	0.459	1.55	238.148	238.066	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 200	0.199	1.48	238.384	238.384	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 203	0.505	1.57	237.981	237.866	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 207	0.795	0.57	237.827	237.818	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 208	0.795	0.57	237.793	237.792	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 218a	0.806	0.58	237.773	237.756	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 209a	0.806	0.58	237.74	237.73	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 213	0.798	0.83	237.687	237.648	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
P 216a	0.8	0.79	237.61	237.608	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
Pipe11344:	0.874	1.42	237.705	237.605	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
P 336	0.363	1.68	238.161	237.598	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P 337	0.366	1.69	237.38	237.278	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
P 338	0.366	2.02	237.137	236.285	AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
Pipe14830:	0.82	1.31	237.774	237.724	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 140	0.456	1.55	237.325	237.245	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
P 131	0.79	1.96	237.482	237.452	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
Pipe10930:	3.381	1.26	238.562	238.512	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2

#### CHANNEL DETAILS

Name	Max Q (cu.m/s)	Max V (m/s)	Due to Storm
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#### OVERFLOW ROUTE DETAILS

Name	Max Q U/S	Max Q D/S	Safe Q	Max D	Max DxV	Max Width	Max V	Due to Storm
F 66	0.001	0.001	0.142	0.029	0.01	0.33	0.26	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 67	0.098	0.098	0.142	0.124	0.07	3.26	0.59	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 22\1	0	0	0.142	0	0	0	0	
F 22\2	0.078	0.078	0.142	0.115	0.07	2.96	0.57	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 373	0	0	0.142	0	0	0	0	
F 374	0.144	0.144	0.142	0.141	0.09	3.83	0.64	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 249	0	0	0.142	0	0	0	0	
OF51143	0	0	0.142	0	0	0	0	
OF51134	0	0	0	0	0	0	0	
OF51139	0	0	0.142	0	0	0	0	
F 239	0.914	0.914	0.142	0.257	0.27	7.02	1.05	AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
F 253	0	0	0.142	0	0	0	0	
F 257	0.82	0.82	0.142	0.248	0.25	6.8	1.02	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 158	0	0	0.142	0	0	0	0	
F 157	1.174	1.174	0.142	0.279	0.32	7.58	1.14	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 433	0	0	0.142	0	0	0	0	
F 445a	0	0	0.142	0	0	0	0	
F 445	0.2	0.2	0.142	0.159	0.1	4.58	0.66	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 443	0	0	0.142	0	0	0	0	
F 120	0	0	0.142	0	0	0	0	
F 121	3.913	3.913	0.142	0.426	0.72	8.85	1.7	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 126	0	0	0.142	0	0	0	0	
F 129	4.079	4.079	0.142	0.433	0.75	8.85	1.73	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 147	0	0	0.142	0	0	0	0	
F 142	1.901	1.901	0.142	0.331	0.43	8.85	1.3	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 93	0	0	0.142	0	0	0	0	
F 94	0.13	0.13	0.142	0.136	0.09	3.68	0.62	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 64	0	0	0.142	0	0	0	0	
F 65	0.348	0.348	0.142	0.189	0.15	5.32	0.77	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 188	0	0	0.142	0	0	0	0	
OF118839	0.569	0.569	0.142	0.22	0.2	6.1	0.91	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F 108	0	0	0.142	0	0	0	0	
F 109	1.483	1.483	0.142	0.303	0.37	8.17	1.22	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 198	0	0	0.142	0	0	0	0	
F 197	1.731	1.731	0.142	0.32	0.41	8.59	1.27	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 178	0	0	0.142	0	0	0	0	
F 177	1.077	1.077	0.142	0.271	0.3	7.39	1.1	AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF51117	0	0	0.142	0	0	0	0	
F 901	0.229	0.229	0.142	0.166	0.11	4.76	0.68	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 40	0	0	0.142	0	0	0	0	
F 41	0.469	0.469	0.142	0.207	0.18	5.77	0.85	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 75	0	0	0.142	0	0	0	0	
F 74	0.138	0.138	0.142	0.138	0.09	3.75	0.64	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 307	0	0	0.142	0	0	0	0	
F 341	0	0	0.142	0	0	0	0	
F 343	0.122	0.122	0.142	0.133	0.08	3.58	0.62	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 4	0	0	0.142	0	0	0	0	
F 393	0	0	0.142	0	0	0	0	
F 392	0.277	0.277	0.142	0.176	0.13	5	0.72	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 3	0	0	0.142	0	0	0	0	
F 394	0	0	0.142	0	0	0	0	
F 110	0	0	0.142	0	0	0	0	
F 111	3.639	3.639	0.142	0.415	0.69	8.85	1.65	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 386	0	0	0.142	0	0	0	0	
OF51147	0	0	0.142	0	0	0	0	
F 385	1.021	1.021	0.142	0.266	0.29	7.26	1.09	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F 376	0	0	0.142	0	0	0	0	
F 377	1.711	1.711	0.142	0.319	0.4	8.57	1.27	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 215	0	0	0.142	0	0	0	0	
F 379	1.345	1.345	0.142	0.293	0.35	7.92	1.18	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 160	0	0	0.142	0	0	0	0	
F 159	1.174	1.174	0.142	0.279	0.32	7.58	1.14	AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F 98a	0	0	0.142	0	0	0	0	
F 99	2.174	2.174	0.142	0.346	0.47	8.85	1.37	AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2

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F446	0	0	0.142	0	0	0	0
F446a	0.435	0.435	0.142	0.202	0.17	5.65	0.83 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F303	0	0	0.142	0	0	0	0
F48	0	0	0.142	0	0	0	0
F49	0.187	0.187	0.142	0.156	0.1	4.5	0.65 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F313	0	0	0.142	0	0	0	0
F314	0.185	0.185	0.142	0.156	0.1	4.5	0.64 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F414	0	0	0.142	0	0	0	0
F415	0.241	0.241	0.142	0.169	0.12	4.82	0.69 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F274	0.767	0.767	0.142	0.242	0.24	6.66	1 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
OF51150	0	0	0.142	0	0	0	0
F327	0.387	0.387	0.142	0.195	0.16	5.46	0.8 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F2\4	0.281	0.281	0.142	0.177	0.13	5.01	0.73 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F71	0.281	0.281	0.142	0.177	0.13	5.01	0.73 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Node15a	0.724	0.724	0.142	0.238	0.23	6.54	0.98 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Node 22\	1.236	1.236	0.142	0.284	0.33	7.71	1.15 AR&R 5 year, 45 minutes storm, average 41.4 mm/h, Zone 2
F Node 31\4	0.615	0.615	0.142	0.225	0.21	6.24	0.93 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F61\2	0	0	0.142	0	0	0	0
F Node 74\z	0.3	0.3	0.142	0.181	0.13	5.11	0.74 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
F Node 93\z	0.061	0.061	0.142	0.106	0.06	2.68	0.54 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F9	0	0	0.142	0	0	0	0
F8	0	0	0.142	0	0	0	0
F6	0	0	0.142	0	0	0	0
F5	0.184	0.184	0.142	0.155	0.1	4.48	0.65 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
FPit12	0	0	0.142	0	0	0	0
FPit13	0.285	0.285	0	0.066	0.03	13.26	0.49 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
FPit32	0	0	0.142	0	0	0	0
OF78192	0	0	0.142	0	0	0	0
FPit29	0	0	0.142	0	0	0	0
FPit34	0.166	0.166	0.142	0.147	0.1	4.05	0.66 AR&R 5 year, 1.5 hours storm, average 26.2 mm/h, Zone 2
FPit GPT	0	0	0.142	0	0	0	0
OF78154	0.906	0.906	0	0.113	0.08	17.95	0.69 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
OF51128	0	0	0	0	0	0	0
F401	0.953	0.953	0.142	0.26	0.28	7.11	1.07 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
FPit6	0	0	0.142	0	0	0	0
FPit8	0.371	0.371	0.142	0.192	0.15	5.41	0.79 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
F371	0	0	0	0	0	0	0
OF153962	0.874	0.874	0.182	0.5	0.44	4	0.87 AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
OF137218	2.269	2.571	0	0.18	0.17	22.81	0.95 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF82141	4.13	4.13	1.753	0.423	0.41	10	0.98 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF82144	3.384	3.384	1.753	0.374	0.34	10	0.9 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
F273	0	0	0.142	0	0	0	0
F347	0	0	3.226	0	0	0	0
F346	0	0	0.142	0	0	0	0
F348	0.518	0.518	0.142	0.213	0.19	5.93	0.88 AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
F291	0	0	0	0	0	0	0
F290	0.218	0.218	0.142	0.164	0.11	4.69	0.67 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF118859	0	0	0.142	0	0	0	0
F279	0.335	0.335	0.142	0.187	0.14	5.27	0.76 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF118882	0.049	0.049	0.142	0.099	0.05	2.44	0.52 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F183	1.077	1.077	0.142	0.271	0.3	7.39	1.1 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF121852	0	0	0	0	0	0	0
OF121886	0.199	0.199	0.142	0.159	0.1	4.58	0.66 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF121872	0	0	0	0	0	0	0
OF121903	0.505	0.505	0.142	0.211	0.19	5.88	0.88 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
OF121924	0	0	1.701	0	0	0	0
OF153967	0	0	0	0	0	0	0
F395	0.961	0.961	0.142	0.261	0.28	7.12	1.07 AR&R 5 year, 3 hours storm, average 15.9 mm/h, Zone 2
OF51123	0	0	0.142	0	0	0	0
F336	0	0	0.142	0	0	0	0
F337	0.01	0.01	0.142	0.06	0.02	1.12	0.41 AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2
F338	0	0	0.142	0	0	0	0
F345	0.369	0.369	0.142	0.192	0.15	5.39	0.79 AR&R 5 year, 6 hours storm, average 9.5 mm/h, Zone 2
OF206891	0	0	0.142	0	0	0	0
OF185927	0.82	0.82	0.142	0.248	0.25	6.8	1.02 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF218457	0.482	0.482	0.142	0.209	0.18	5.81	0.86 AR&R 5 year, 30 minutes storm, average 52.4 mm/h, Zone 2
OF135437	1.471	1.471	1.753	0.224	0.15	10	0.66 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146104	1.445	1.445	0.142	0.3	0.36	8.1	1.21 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146119	1.901	1.901	0.142	0.331	0.43	8.85	1.3 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146085	1.111	1.111	0.142	0.274	0.31	7.46	1.12 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF146067	1.901	1.901	0.142	0.331	0.43	8.85	1.3 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2
OF149269	0	0	0	0	0	0	0
OF205948	3.381	3.381	1.753	0.374	0.34	10	0.9 AR&R 5 year, 1 hour storm, average 34.5 mm/h, Zone 2

#### DETENTION BASIN DETAILS

Name	Max WL	MaxVol	Max Q	Max Q	Max Q	Total	Low Level	High Level
SchoolOval	237.29	1186.1	0.82	0.82	0	0		
PaytenParl	238.2	11136.1	0.906	0	0.906	0		
DundasPar	238.19	13305.6	0.953	0.953	0	0		
U/S Railwa	238.02	2070.2	0.874	0.874	0	0		
Proposed_	238.56	19915.9	0.363	0.363	0	0		
Proposed_	237.98	11534.7	0.82	0.82	0	0		

CONTINUITY CHECK for AR&R 5 year, 2 hours storm, average 21.4 mm/h, Zone 2

Node	Inflow	Outflow	Storage	Ch	Difference
	(cu.m)	(cu.m)	(cu.m)	%	
66	707.11	707.35	0	0	
67	707.35	707.35	0	0	
22\1	371.87	371.47	0	0.1	
22\2	371.47	371.47	0	0	
373	788.1	787.52	0	0.1	
374	787.52	787.52	0	0	
249	4027.76	3766.86	0	6.5	

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248	3766.86	3749.62	0	0.5
244	7986.9	7947.04	0	0.5
243	7947.04	7937.61	0	0.1
242	7937.61	7927.09	0	0.1
239	7927.09	7926.84	0	0
253	0	0	0	0
257	6658.75	6477.08	0	2.7
158	11081.4	11064.87	0	0.1
157	11064.87	11064.02	0	0
433	713.47	712.4	0	0.1
445a	1314.75	1312.13	0	0.2
445	1312.13	1312.1	0	0
443	603.3	602.35	0	0.2
120	2614.39	2613.33	0	0
121	32616.84	32431.55	0	0.6
126	1546.31	1545.4	0	0.1
129	33976.93	33922.46	0	0.2
147	956.04	954.75	0	0.1
142	15441.24	15394.27	0	0.3
93	867.52	867.1	0	0
94	867.1	867.1	0	0
64	2882.41	2881.45	0	0
65	2881.45	2881.42	0	0
188	5526.59	5521.09	0	0.1
183	5521.09	5520.79	0	0
108	468.95	468.61	0	0.1
109	14400.99	14375.81	0	0.2
198	192.76	192.58	0	0.1
197	14503.77	14486.53	0	0.1
178	10448.92	10431.98	0	0.2
177	10431.98	10431.37	0	0
219	1393.97	1393.17	0	0.1
901	1393.17	1393.17	0	0
40	3104.36	3103.43	0	0
41	3103.43	3103.43	0	0
75	754.58	754.14	0	0.1
74	754.14	754.14	0	0
307	700.43	700.18	0	0
304	700.18	700.18	0	0
341	481.97	480.83	0	0.2
342	480.83	479.12	0	0.4
343	479.12	479.12	0	0
4	1773.04	1772.61	0	0
393	1772.61	1771.28	0	0.1
392	1771.28	1771.28	0	0
3	0	0	0	0
394	0	0	0	0
110	14375.81	14367.57	0	0.1
111	30017.81	30003.49	0	0
386	6110.25	6102.86	0	0.1
387	6102.86	6103.27	0	0
385	6103.27	6103.22	0	0
376	787.52	787.12	0	0.1
377	14322.45	14311.2	0	0.1
215	1171.64	1167.99	0	0.3
217a	9171.25	9167.06	0	0
214a	9167.06	9143.34	0	0.3
379	12143.02	12142.28	0	0
160	695.73	695.33	0	0.1
159	11126.69	11081.4	0	0.4
98a	5514.69	5507.78	0	0.1
99	15706.38	15650.26	0	0.4
446	1312.1	1311.46	0	0
446a	2869.4	2869.38	0	0
303	906.19	905.73	0	0.1
308a	905.73	905.73	0	0
48	1521.3	1520.52	0	0.1
49	1520.52	1520.5	0	0
313	1124.57	1124.3	0	0
314	1124.3	1124.3	0	0
414	1559.37	1557.95	0	0.1
415	1557.95	1557.95	0	0
276	3649.17	3641.14	0	0.2
275	6090.41	6069.1	0	0.3
274	6069.1	6068.91	0	0
325	3153.57	3152.96	0	0
326	3152.96	3151.32	0	0.1
327	3151.32	3151.29	0	0
OUTLET 05	1967.06	1967.06	0	0
2\4	1751.54	1751.54	0	0
71	1751.54	1751.54	0	0
Node15a	5340.29	5340.04	0	0
Outlet 67	5340.04	5340.04	0	0
Node 22\3	8616.96	8584.38	0	0.4
Node 314a	3993.67	3992.85	0	0
OUTLET 34	5216.88	5216.88	0	0
OUTLET 38	6103.22	6103.22	0	0
OUTLET 41	3103.43	3103.43	0	0
OUTLET 49	1520.5	1520.5	0	0
61\2	0	0	0	0
Node 74a	1692.88	1692.88	0	0
OUTLET 74	1692.88	1692.88	0	0
Node 93a	434.72	434.72	0	0
OUTLET 12	49212.79	49212.79	0	0
9	1947.49	1943.27	0	0.2

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8	1976.85	1972.95	0	0.2
6	1972.95	1967.27	0	0.3
5	1967.27	1967.06	0	0
Pit12	3138.48	3136.54	0	0.1
Pit13	3134.24	3125.9	0	0.3
Pit32	3125.9	3115.62	0	0.3
SchoolOval	6715.5	6715.47	0.02	0
Pit29	6715.47	6704.45	0	0.2
Pit34	8557.82	8557.49	0	0.2
Pit GPT	7618.74	7607.89	0	0.1
discharge f	7607.89	7607.89	0	0
O Pit GPT	0	0	0	0
O 392	1771.28	1771.28	0	0
PaytenParl	5621.12	0	5425.91	3.5
DundasPar	14660.95	2459.33	11781.85	2.9
401	2459.33	2455.43	0	0.2
Pit6	0	-0.41	0	0
Pit8	-0.41	-2.3	0	0
U/S Railwa	9739.76	7753.56	1804.48	1.9
N192692	7753.56	7749.02	0	0.1
N172535	11210.77	11210.77	0	0
N101397	33292.21	32794.09	0	1.5
N101398	19253.74	19253.74	0	0
273	2451.86	2449.27	0	0.1
347	1268.42	1267.83	0	0
346	1871.48	1871.85	0	0
348	5347.22	5216.88	0	2.4
291	2119.88	2118.57	0	0.1
290	2118.57	2118.46	0	0
278	3259.1	3257.08	0	0.1
279	3257.08	3256.92	0	0
192	4936.19	4932.95	0	0.1
183a	10453.74	10448.92	0	0
200	2028.9	2027.45	0	0.1
199	2027.45	2027.31	0	0
203	5150.99	5145.81	0	0.1
202	5145.81	5145.44	0	0
207	8097.78	8088.25	0	0.1
208	8088.25	8084.55	0	0
218a	8084.55	8069.23	0	0.2
209a	8069.23	8051.83	0	0.2
213	8051.83	8028.8	0	0.3
216a	8028.8	8003.26	0	0.3
HW1103	7749.02	7677.39	0	0.9
N192689	8496.32	8491.76	0	0.1
N162485	32794.09	32794.09	0	0
61\1	0	0	0	0
Proposed_	19347.24	2734.64	16557.56	0.3
337	3087.57	3073.71	0	0.4
338	3012.6	2998.33	0	0.5
345	3477.44	3475.36	0	0.1
Proposed_	16511.23	6662.86	9818.09	0.2
N230386	6662.86	6658.75	0	0.1
N270468	2176.58	2176.58	0	0
N168004	10198.6	10198.6	0	0
140	15348.34	15291.78	0	0.4
141	15291.76	15290.37	0	0
131	15394.27	15349.57	0	0.3
130	15349.59	15348.34	0	0
HW47	19253.74	19105.95	0	0.8
N184609	19105.95	19104.87	0	0

Run Log for 055 run at 15:41:02 on 14/4/2021 using version 2020.061

No water upwelling from any pit.

Freeboard was less than 0.15m at 243, Pit8, Pit13, 248, 393

The maximum flow in these overflow routes is unsafe: OF205948, OF146119, OF146104, OF146085, OF146067, OF218457, OF185927, OF153962, OF121903, OF121886, OF118839, F 279, F 290, OF82144, OF82

These overflow routes carried water uphill (adding energy): F Pit8. These results may be invalid. This is likely due to either incorrect surface levels specified at pits or high downstream tailwater levels which th

IGNORE THESE WARNINGS AT YOUR OWN PERIL.\cf1

## **Attachment No. 3**

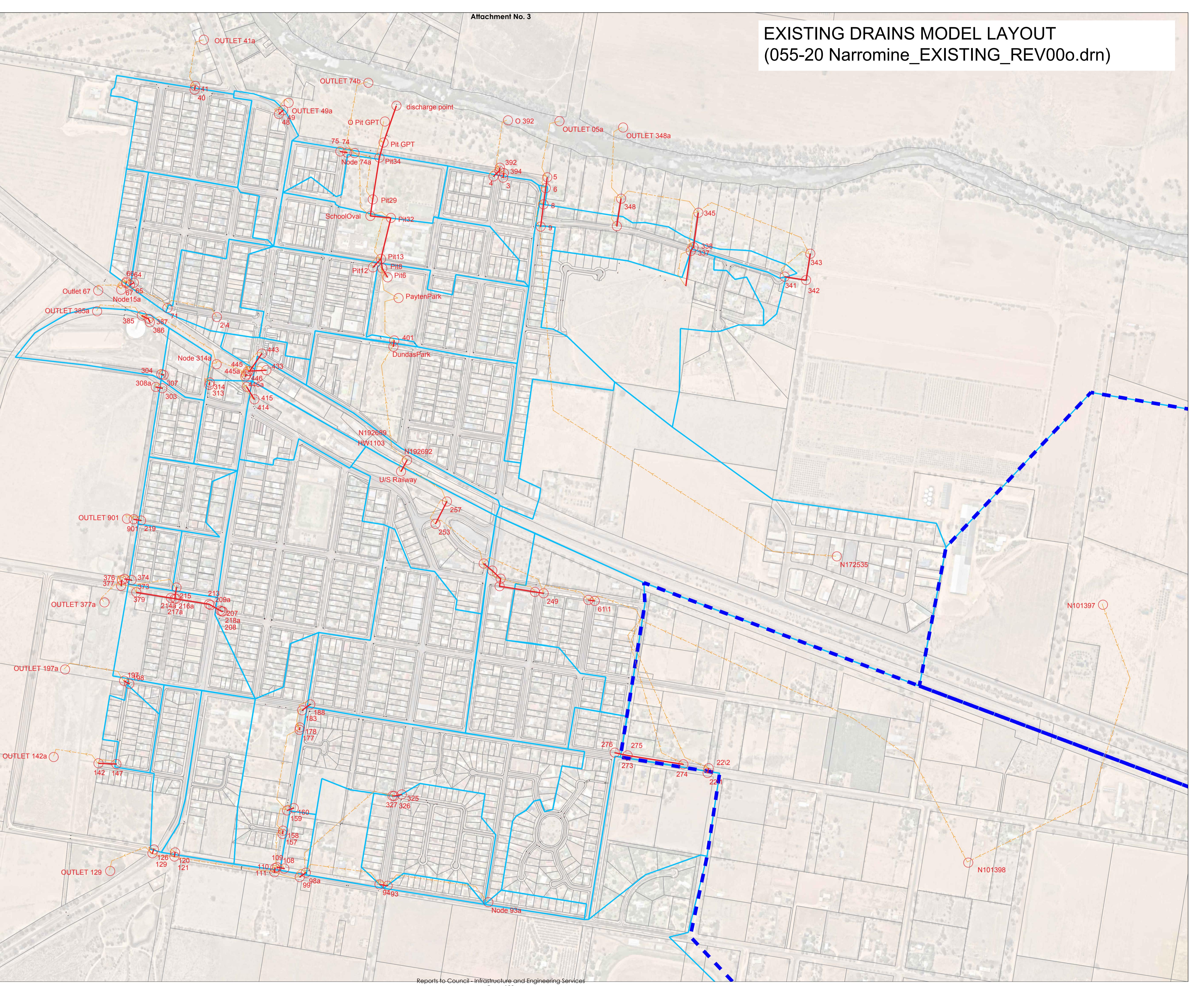
141, OF137218, OF78154, F 5, F 142, F 157, F 109, F 177, F 159, F 183, F 197, F 2\4, F 71, F Node15a, F 274, F 239, F 257, F Node 22\3, F 314, F Node 314a, F 327, F 345, F 348, F 374, F 379, F 377, F 385, F 392, F  
ie Lite Hydraulic model cannot handle. Analysing the latter requires solving the full unsteady flow equations in overflow routes using the Full Unsteady Hydraulic model (Formerly Premium Hydraulic model).

### **Attachment No. 3**

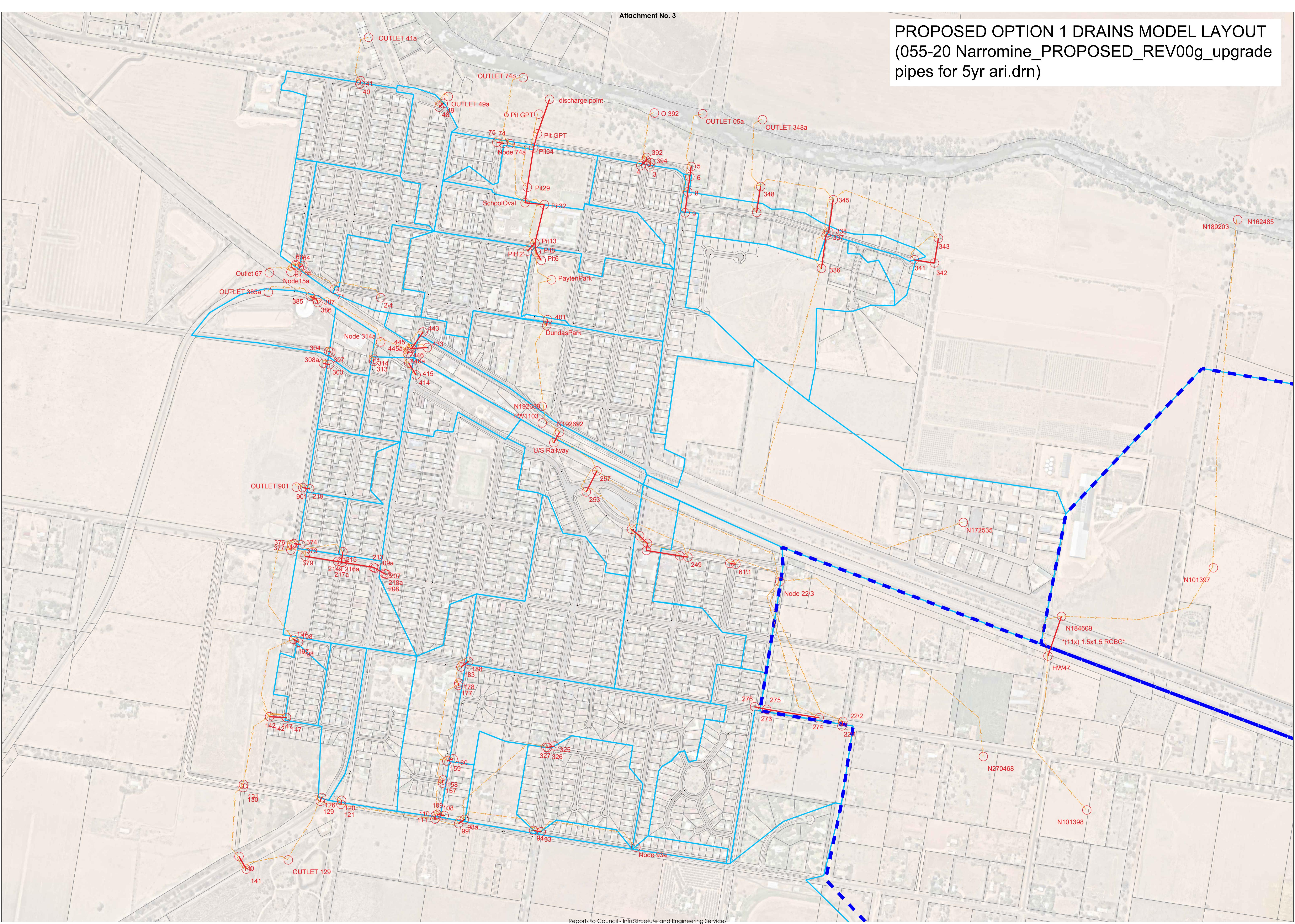
:95, F 401, F 41, F 415, F 446a, F 445, F 49, F 65, F Node 74a, F 901, F 99, F 111, F 121, F 129, F Pit34, F Pit13, F Pit8

## **Appendix C – DRAINS Model Layout**

# EXISTING DRAINS MODEL LAYOUT (055-20 Narromine\_EXISTING\_REV00o.drn)



**PROPOSED OPTION 1 DRAINS MODEL LAYOUT**  
**(055-20 Narromine\_PROPOSED\_REV00g\_upgrade**  
**pipes for 5yr ari.drn)**



PROPOSED OPTION 2 DRAINS MODEL LAYOUT  
(055-20 Narromine\_PROPOSED\_REV00g\_upgrade  
pipes for 5yr ari with basin.drn)

